

**COMPLETE THIS FORM WHEN DROPPING OFF ANY
CORRESPONDENCE AND/OR PLANS TO THE HOWARD COUNTY
DEPARTMENT OF INSPECTIONS, LICENSES AND PERMITS COUNTER:**

Date: 8-9-23 ONLINE SUBMITTAL PAPER SUBMITTAL

To: Plan Review
 (Reviewer/Requestor's Name) (Division)

From: DUSTIN WATSON INPLACE DESIGN 410 235-0100
 (Your Name, Company Name) (Phone Number)

Subject: Project name BROOKE RES. ACCESSORY BLDG.
 Project site address 12419 ALL DAUGHTERS LANE
 Permit # B23000244 SDP # _____
 Other information pertinent to this project _____

Please check the attachments below that you are submitting with this transmittal:

- Letter of response to address plan review comment letter
- Revised plans and/or revised details: When submitting for a complete re-review, **duplicate sets shall be submitted.**
- Letter Summarizing Changes
- Energy conservation calculations
- Copies of Plot Plan, ARCH PLANS (be specific).
 _____ Health Department Request _____ DPZ/ DED Request _____ Applicant's Request
- Two sets of single-family model plans to be placed on permanent file: Model name and/or # _____
- Other _____

*Based on uploaded/revised plot plan
on 8/23/23, OK for approval. closed
gjs 8/28/23*

Contact Person Information: (Required)

DUSTIN WATSON Telephone No: 410 235-0100
 Please Print Name

E-Mail Address: DWATSON@INPLACE-DESIGN.COM

PLEASE ASSURE ALL DOCUMENTS AND/OR REVISIONS ARE APPROPRIATELY SIGNED AND SEALED, IF NECESSARY, BY A LICENSED ARCHITECT OR ENGINEER. PLEASE BE ADVISED THAT INSUFFICIENT INFORMATION MAY RESULT IN THE DELAY OF REVIEW BY THE PLANS EXAMINER. THE DEPARTMENT OF INSPECTIONS, LICENSES AND PERMITS WILL CONTACT YOU IF THERE IS A PROBLEM. IN ADDITION, ONCE THE BUILDING PERMIT IS APPROVED BY THE PLAN REVIEW DIVISION AND ALL OTHER REQUIRED SIGNATORY AGENCIES, AND THE BUILDING PERMIT IS READY FOR ISSUANCE, THE PERMIT DIVISION WILL NOTIFY THE APPROPRIATE CONTACT PERSON FOR PERMIT PICK UP. ALL PERMIT STATUS INQUIRIES SHALL BE DIRECTED TO THE PERMIT DIVISION AT 410-313-2455 OPTION #4 OR BY VISITING MYHOWARD.INFO. CODE RELATED QUESTIONS AND PLAN REVIEW INQUIRIES SHALL BE DIRECTED TO THE PLAN REVIEW DIVISION AT 410-313-2436. PLEASE ALLOW A MINIMUM OF FIVE (5) WORKING DAYS FOR ANY PLAN SUBMITTALS TO BE REVIEWED. THANK YOU.

Received by JR
 White-Plan Review / Yellow-Applicant / Pink-Permit Division
 t:\Operations\Updated forms\HoCoTransmittalForm05.2022

Rev #1

RECEIVED

AUG 09 2023

LICENSES & PERMITS
DIVISION

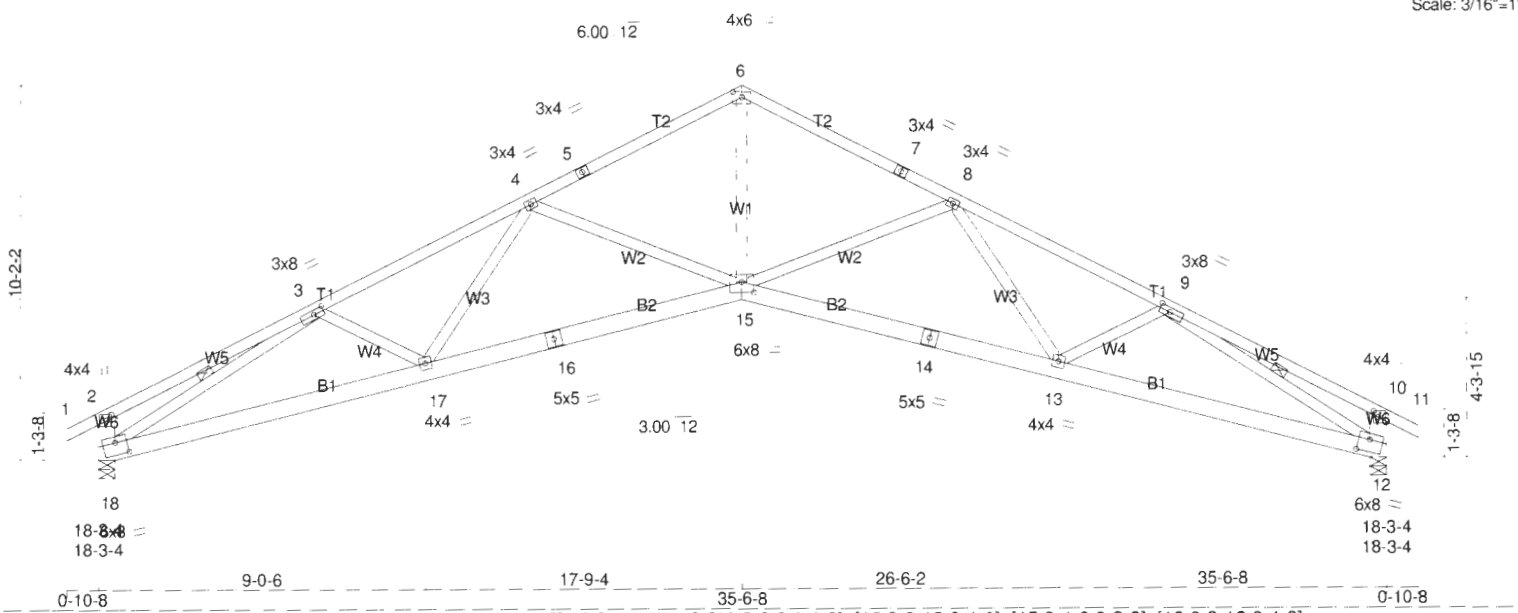


Plate Offsets (X,Y) -- [2.0-2.0,0-1.8], [3.0-3.8,0-1.8], [6.0-3.0,0-1.12], [9.0-3.8,0-1.8], [10.0-2.0,0-1.8], [12.0-3.12,0-4.0], [15.0-4.0,0-3.8], [18.0-3.12,0-4.0]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 30.0	2-0-0	TC 0.60	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 27.7/40.0	Plate Grip DOL 1.15	BC 0.64	Vert(LL) -0.31 13-15 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.79	Vert(CT) -0.53 13-15 >794 180		
BCLL 0.0	Rep Stress Incr YES	Matrix-R	Horz(CT) 0.38 12 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 223 lb	FT = 0%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 2-6-7 oc purlins, except end verticals.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 7-5-2 oc bracing.
WEBS 2x4 SP No.2 *Except* W6: 2x6 SP No.1	WEBS 1 Row at midpt 3-18, 9-12
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 18=1757/0-5-8 (min. 0-2-1), 12=1757/0-5-8 (min. 0-2-1)
 Max Horz 18=-167(LC 10)
 Max Uplift 18=-532(LC 12), 12=-532(LC 13)
 Max Grav 18=1843(LC 2), 12=1843(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-19=-794/276, 3-19=-678/296, 3-20=-3970/1140, 4-20=-3863/1152, 4-5=-3328/843,
 5-21=-3224/851, 6-21=-3223/862, 6-22=-3223/886, 7-22=-3224/874, 7-8=-3328/866,
 8-23=-3863/1054, 9-23=-3970/1043, 9-24=-678/286, 10-24=-794/266, 2-18=-650/296,
 10-12=-650/291
 BOT CHORD 18-25=-1122/3452, 17-25=-1101/3486, 16-17=-926/3540, 16-26=-913/3542, 15-26=-903/3567,
 15-27=-669/3567, 14-27=-679/3542, 13-14=-692/3540, 13-28=-887/3486, 12-28=-908/3452
 WEBS 6-15=-556/2421, 8-15=-748/461, 4-15=-748/450, 3-18=-3359/961, 9-12=-3359/881

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16: Vult=115mph (3-second gust) Vasd=91mph; TCDL=4.2psf; BCDL=6.0psf; h=30ft; Cat. II; Exp C; Pr. Enclosed; MWFRS (envelope) gable end zone: cantilever left and right exposed : end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) TCLL: ASCE 7-16: Pr=30.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=40.0 psf; Ps=27.7 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Unbalanced snow loads have been considered for this design.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 27.7 psf on overhangs non-concurrent with other live loads.
 - 7) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 10) Bearing at joint(s) 18, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 18 and 532 lb uplift at joint 12.

Job	Truss	Truss Type	Qty	Ply
Q-90266H	T1S	SCISSORS	18	1

Job Reference (optional)

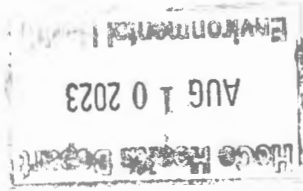
SMT LLC, Jarrettsville, MD 21084

Run: 8,510 s Nov 15 2021 Print: 8,510 s Nov 15 2021 MiTek Industries, Inc. Thu Jul 27 13:33:16 2023 Page 2
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NOTES-

- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) This truss has been designed for a moving concentrated load of 50.0lb live located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard



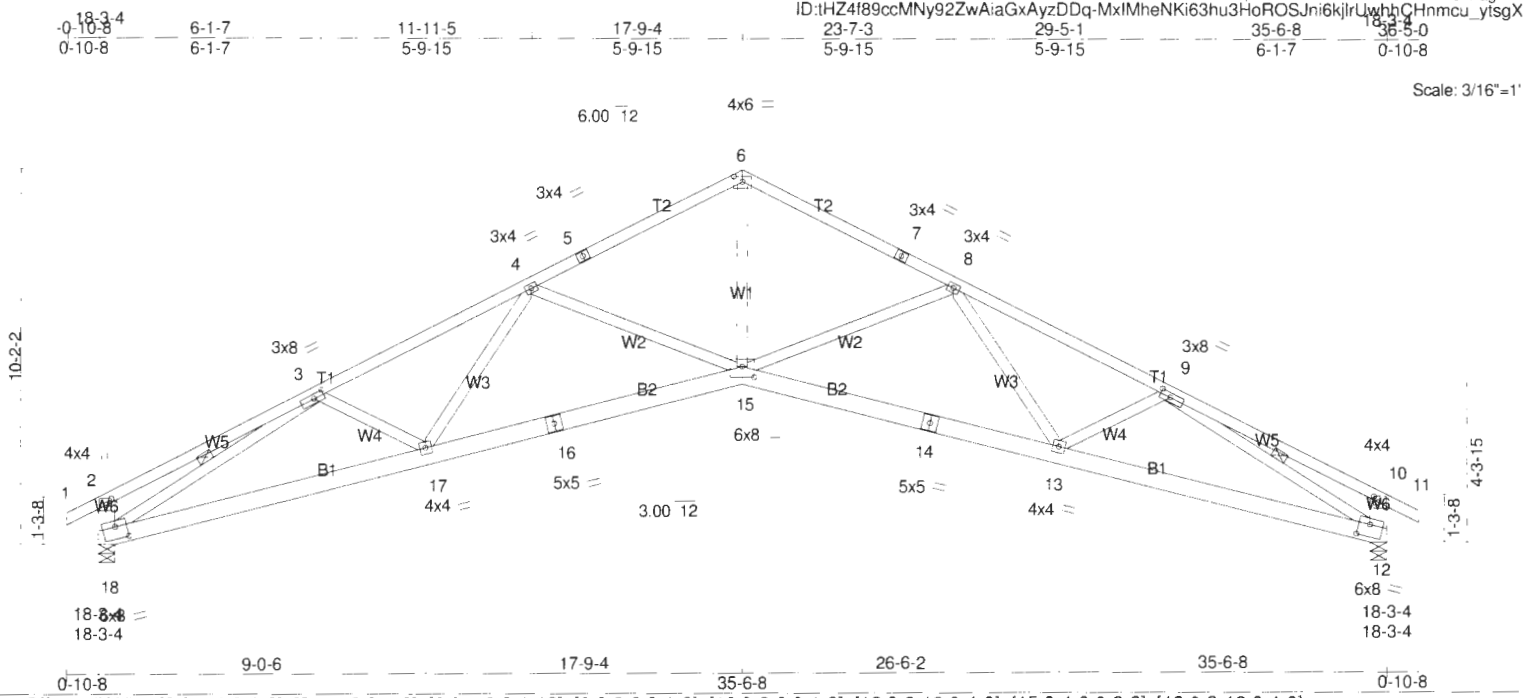


Plate Offsets (X, Y) -- [2:0-2-0-0-1-8], [3:0-3-8-0-1-8], [6:0-3-0-0-1-12], [9:0-3-8-0-1-8], [10:0-2-0-0-1-8], [12:0-3-12-0-4-0], [15:0-4-0-0-3-8], [18:0-3-12-0-4-0]

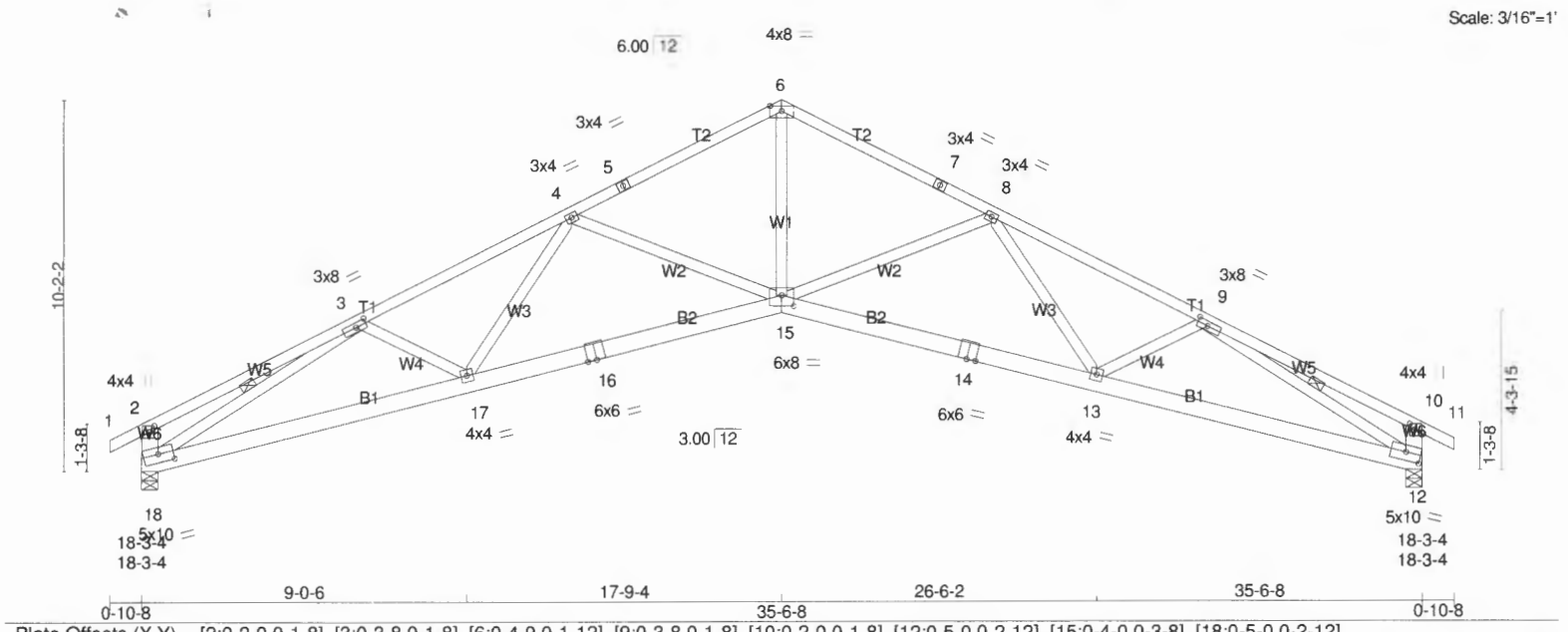
LOADING (psf)	SPACING-	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 30.0	2-0-0	TC 0.60	Vert(LL)	-0.31	13-15	>999	MT20	244/190
Snow (Ps/Pg) 27.7/40.0	Plate Grip DOL 1.15	BC 0.64	Vert(CT)	-0.53	13-15	>794		
TCDL 10.0	Lumber DOL 1.15	WB 0.79	Horz(CT)	0.38	12	n/a		
BCLL 0.0	Rep Stress Incr YES	Matrix-R						
BCDL 10.0	Code IRC2018/TPI2014						Weight: 223 lb	FT = 0%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 2-6-7 oc purlins, except end verticals.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 7-5-2 oc bracing.
WEBS 2x4 SP No.2 *Except* W6: 2x6 SP No.1	WEBS 1 Row at midpt 3-18, 9-12
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 18=1757/0-5-8 (min. 0-2-1), 12=1757/0-5-8 (min. 0-2-1)
 Max Horz 18=-167(LC 10)
 Max Uplift 18=532(LC 12), 12=532(LC 13)
 Max Grav 18=1843(LC 2), 12=1843(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-19=-794/276, 3-19=-678/296, 3-20=-3970/1140, 4-20=-3863/1152, 4-5=-3328/843,
 5-21=-3224/851, 6-21=-3223/862, 6-22=-3223/886, 7-22=-3224/874, 7-8=-3328/866,
 8-23=-3863/1054, 9-23=-3970/1043, 9-24=-678/286, 10-24=-794/266, 2-18=-650/296,
 10-12=650/291
 BOT CHORD 18-25=-1122/3452, 17-25=-1101/3486, 16-17=-926/3540, 16-26=-913/3542, 15-26=-903/3567,
 15-27=-669/3567, 14-27=-679/3542, 13-14=-692/3540, 13-28=-887/3486, 12-28=-908/3452
 WEBS 6-15=-556/2421, 8-15=-748/461, 4-15=-748/450, 3-18=-3359/961, 9-12=-3359/881

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCCL=4.2psf; BCCL=6.0psf; h=30ft; Cat. II; Exp C; Pr. Enclosed; MWFRS (envelope) gable end zone: cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
 - 3) TCLL: ASCE 7-16; Pr=30.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=40.0 psf; Ps=27.7 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) Unbalanced snow loads have been considered for this design.
 - 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 27.7 psf on overhangs non-concurrent with other live loads.
 - 7) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 10) Bearing at joint(s) 18, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 532 lb uplift at joint 18 and 532 lb uplift at joint 12.



LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 30.0	2-0-0	TC 0.85	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 27.7/40.0	Plate Grip DOL 1.15	BC 0.70	Vert(LL) -0.31 13-15 >999 240		
TCDL 10.0	Lumber DOL 1.15	WB 0.87	Vert(CT) -0.60 13-15 >706 180		
BCLL 0.0 *	Rep Stress Incr YES	Matrix-R	Horz(CT) 0.42 12 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 223 lb	FT = 0%

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 9-0-6 oc bracing.
WEBS 2x4 SP No.2 *Except* W4: 2x6 SP No.1	WEBS 1 Row at midpt 3-18, 9-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 18=1917/0-5-8 (min. 0-2-4), 12=1917/0-5-8 (min. 0-2-4)
 Max Horz 18=-167(LC 10)
 Max Uplift 18=-373(LC 12), 12=-373(LC 13)
 Max Grav 18=2002(LC 2), 12=2002(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-19=-847/232, 3-19=-728/243, 3-20=-4442/669, 4-20=-4335/680, 4-5=-3883/288,
 5-21=-3778/295, 21-22=-3763/297, 6-22=-3624/377, 6-23=-3624/402, 23-24=-3763/324,
 7-24=-3778/320, 7-8=-3883/312, 8-25=-4335/582, 9-25=-4442/571, 9-26=-728/233,
 10-26=-847/222, 2-18=-678/267, 10-12=-678/262
BOT CHORD 18-27=-758/3819, 17-27=-735/3853, 16-17=-439/4026, 16-28=-426/4029, 15-28=-416/4054,
 15-29=-191/4054, 14-29=-192/4029, 13-14=-205/4026, 13-30=-520/3853, 12-30=-543/3819
WEBS 6-15=-325/2652, 8-15=-737/473, 9-13=0/304, 4-15=-737/462, 3-17=0/290, 3-18=-3738/583,
 9-12=-3738/502

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=4.2psf; BCDL=6.0psf; h=30ft; Cat. II; Exp C; Pr. Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
 - TCLL: ASCE 7-16; Pr=30.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=40.0 psf; Ps=27.7 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 27.7 psf on overhangs non-concurrent with other live loads.
 - As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 18, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 373 lb uplift at joint 18 and 373 lb uplift at joint 12.

Job	Truss	Truss Type	Qty	Ply	Job Reference (optional)
Q-90266H	T1SA	SCISSORS	4	1	

SMT LLC, Jarrettsville, MD 21084

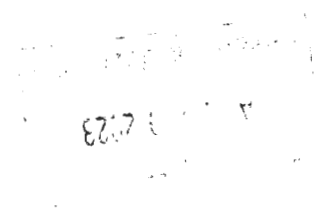
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NOTES-

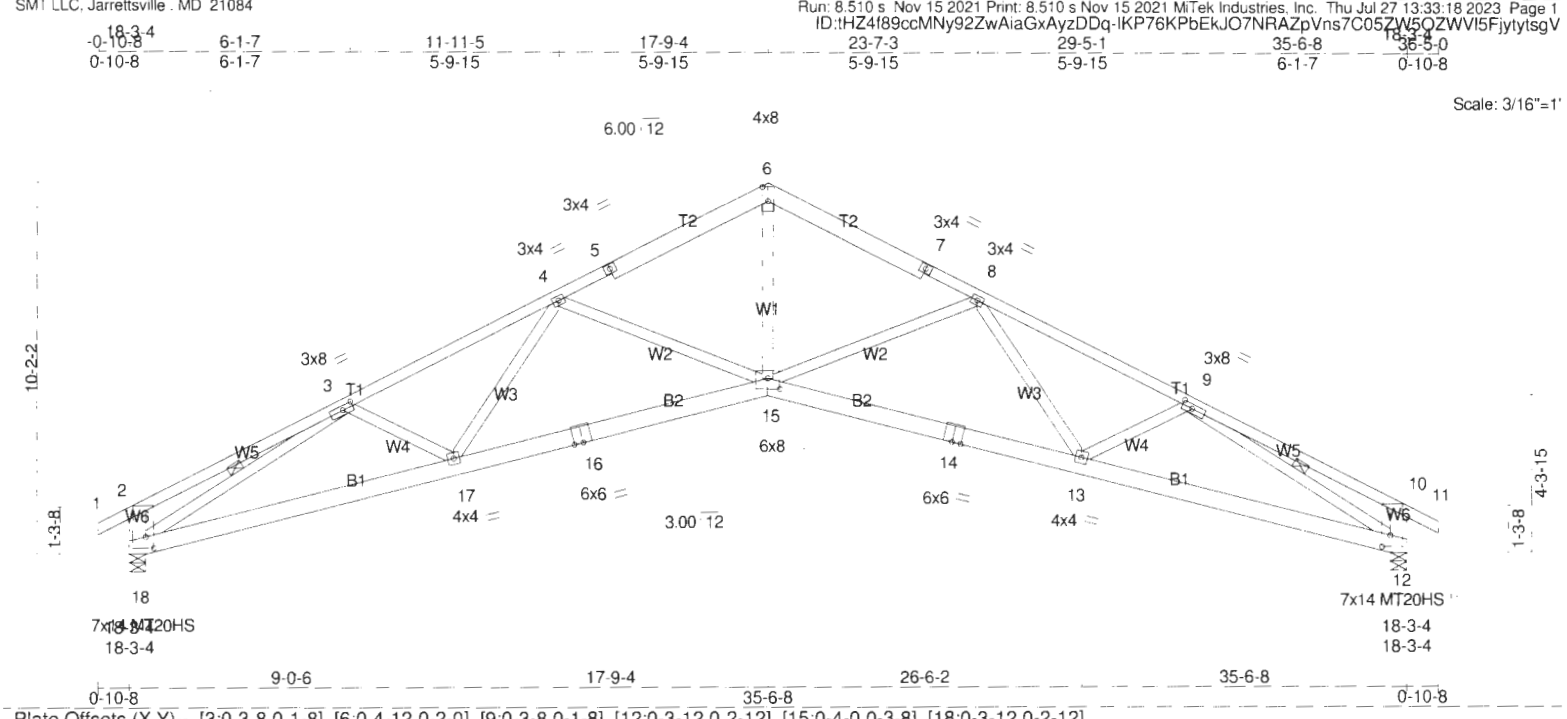
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) This truss has been designed for a moving concentrated load of 50.0lb live located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-2=-75, 2-6=-75, 6-10=-75, 10-11=-75, 15-18=-20, 12-15=-20
- Concentrated Loads (lb)
 - Vert: 22=-159 23=-159



 8/27/23



LOADING (psf)	SPACING-	CSL	DEFL.	PLATES	GRIP
TCLL (roof) 30.0	2-0-0	TC 0.87	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 27.7/40.0	Plate Grip DOL 1.15	BC 0.76	Vert(LL) -0.29 13-15 >999 240	MT20HS	187/143
TCDL 10.0	Lumber DOL 1.15	WB 0.96	Vert(CT) -0.62 13-15 >680 180		
BCLL 0.0	Rep Stress Incr YES	Matrix-R	Horz(CT) 0.45 12 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 231 lb	FT = 0%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1 *Except* T1: 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.2 *Except* W6: 2x6 SP No.1	WEBS 1 Row at midpt 3-18, 9-12
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 18=2057/0-5-8 (min. 0-2-7), 12=2057/0-5-8 (min. 0-2-7)
 Max Horz 18=-166(LC 10)
 Max Uplift 18=-233(LC 12), 12=-233(LC 13)
 Max Grav 18=2142(LC 2), 12=2142(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-19=-882/188, 3-19=-764/199, 3-20=-4854/253, 4-20=-4747/264, 4-5=-4426/0,
 5-21=-4322/0, 21-22=-4298/0, 6-22=-4112/0, 6-23=-4112/0, 23-24=-4298/0, 7-24=-4322/0,
 7-8=-4426/0, 8-25=-4747/167, 9-25=-4854/156, 9-26=-764/189, 10-26=-882/178,
 2-18=-693/244, 10-12=-693/239
 BOT CHORD 18-27=-430/4152, 17-27=-407/4186, 16-17=-13/4433, 16-28=0/4436, 15-28=0/4461,
 15-29=0/4461, 14-29=0/4436, 13-14=0/4433, 13-30=-195/4186, 12-30=-218/4152
 WEBS 6-15=-152/2755, 8-15=-601/518, 9-13=0/368, 4-15=-601/505, 3-17=0/354, 3-18=-4093/243,
 9-12=-4093/164

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=4.2psf; BCDL=6.0psf; h=30ft; Cat. II; Exp C; Pr. Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
 - TCLL: ASCE 7-16; Pr=30.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=40.0 psf; Ps=27.7 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 27.7 psf on overhangs non-concurrent with other live loads.
 - As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 18, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 18 and 233 lb uplift at joint 12.

Scale: 3/16"=1'

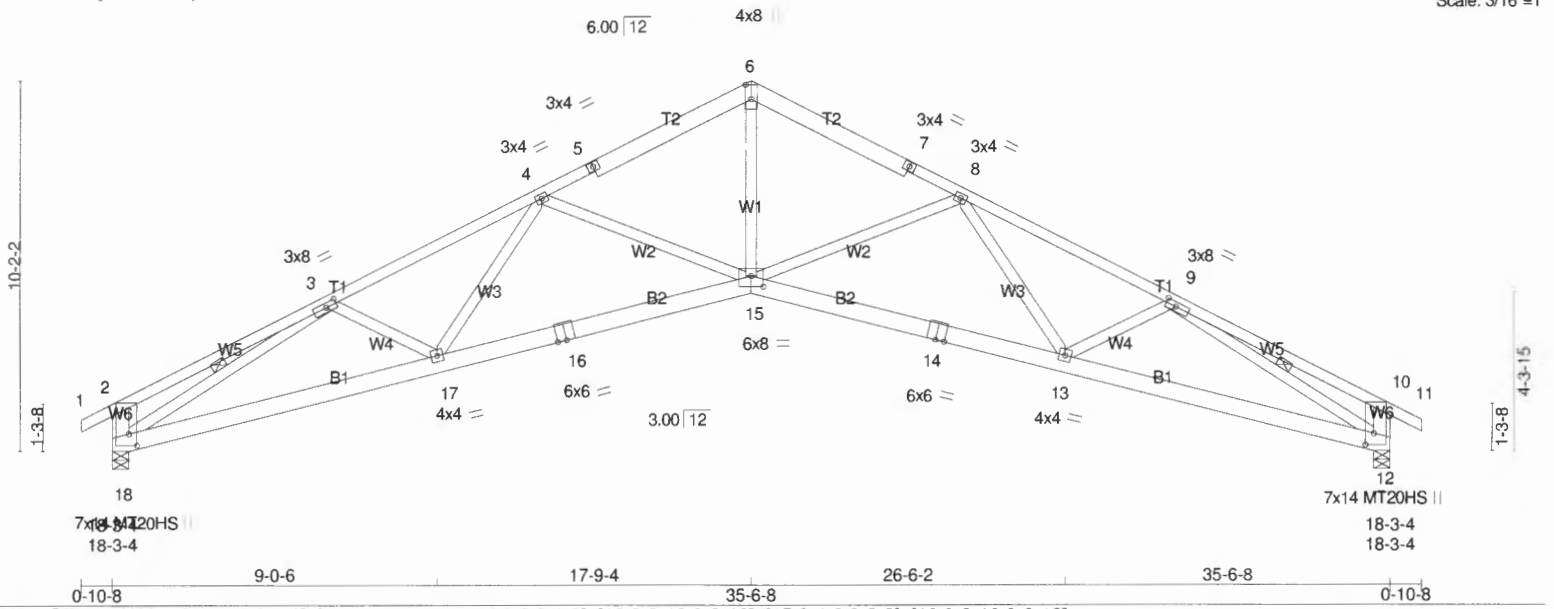


Plate Offsets (X,Y)-- [3:0-3-8,0-1-8], [6:0-4-12,0-2-0], [9:0-3-8,0-1-8], [12:0-3-12,0-2-12], [15:0-4-0,0-3-8], [18:0-3-12,0-2-12]

LOADING (psf)	SPACING-	CSI.	DEFL.	PLATES	GRIP
TCLL (roof) 30.0	2-0-0	TC 0.87	in (loc) l/defl L/d	MT20	244/190
Snow (Ps/Pg) 27.7/40.0	Plate Grip DOL 1.15	BC 0.76	Vert(LL) -0.29 13-15 >999 240	MT20HS	187/143
TCDL 10.0	Lumber DOL 1.15	WB 0.96	Vert(CT) -0.62 13-15 >680 180		
BCLL 0.0	Rep Stress Incr YES	Matrix-R	Horz(CT) 0.45 12 n/a n/a		
BCDL 10.0	Code IRC2018/TPI2014			Weight: 231 lb	FT = 0%

LUMBER-
 TOP CHORD 2x6 SP No.1 *Except*
 T1: 2x4 SP No.1
 BOT CHORD 2x6 SP No.1
 WEBS 2x4 SP No.2 *Except*
 W6: 2x6 SP No.1

BRACING-
 TOP CHORD Structural wood sheathing directly applied, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 3-18, 9-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 18=2057/0-5-8 (min. 0-2-7), 12=2057/0-5-8 (min. 0-2-7)
 Max Horz 18=-166(LC 10)
 Max Uplift 18=-233(LC 12), 12=-233(LC 13)
 Max Grav 18=2142(LC 2), 12=2142(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-19=-882/188, 3-19=-764/199, 3-20=-4854/253, 4-20=-4747/264, 4-5=-4426/0,
 5-21=-4322/0, 21-22=-4298/0, 6-22=-4112/0, 6-23=-4112/0, 23-24=-4298/0, 7-24=-4322/0,
 7-8=-4426/0, 8-25=-4747/167, 9-25=-4854/156, 9-26=-764/189, 10-26=-882/178,
 2-18=-693/244, 10-12=-693/239
 BOT CHORD 18-27=-430/4152, 17-27=-407/4186, 16-17=-13/4433, 16-28=0/4436, 15-28=0/4461,
 15-29=0/4461, 14-29=0/4436, 13-14=0/4433, 13-30=-195/4186, 12-30=-218/4152
 WEBS 6-15=-152/2755, 8-15=-601/518, 9-13=0/368, 4-15=-601/505, 3-17=0/354, 3-18=-4093/243,
 9-12=-4093/164

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=4.2psf; BCDL=6.0psf; h=30ft; Cat. II; Exp C; Pr. Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
 - TCLL: ASCE 7-16; Pr=30.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=40.0 psf; Ps=27.7 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 27.7 psf on overhangs non-concurrent with other live loads.
 - As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 18, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 18 and 233 lb uplift at joint 12.

Job	Truss	Truss Type	Qty	Ply
Q-90266H	T1SB	SCISSORS	8	1

Job Reference (optional)

SMT LLC, Jarrettsville, MD 21084

Run: 8.510 s Nov 15 2021 Print: 8.510 s Nov 15 2021 MiTek Industries, Inc. Thu Jul 27 13:33:18 2023 Page 2
ID:tHZ4f89ccMNy92ZwAiaGxAyzDDq-lKP76KPBekJO7NRAZpVns7C05ZW5OZWV15FjtytsgV

NOTES-

- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) This truss has been designed for a moving concentrated load of 50.0lb live located at all mid panels and at all panel points along the Top Chord and Bottom Chord, nonconcurrent with any other live loads.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
 - Vert: 1-2=-75, 2-6=-75, 6-10=-75, 10-11=-75, 15-18=-20, 12-15=-20
- Concentrated Loads (lb)
 - Vert: 22=-300 23=-300

AUG 12 2023



inPLACE Design LLC
2002 Clipper Park Road
Suite 110A
Baltimore, MD 21211 USA

Date: August 8, 2023

To: Howard County
Inspections, Licenses & Permits
3430 Courthouse Drive
Ellicott City, MD 21043

App: Plan Reviewers
RE: Building Review

Permit No: B23000244
Project Name: Brooke – Residential Accessory Structure

Per previous submittal our variance was denied. This resubmission reduces the footprint, removes the mezzanine, and restroom.

Attention: Michael Blevins - Building Review

Comment:

1. *How will bathroom meet 2018 IECC Insulation requirements, how will it be heated to prevent freezing pipes?*
 - a. The bathroom has been removed.
 2. *Structural drawings say Engineers stamp is void unless green seal is present, no green seal.*
 - a. Green seal removed.
 3. Roof trusses should be designed for 40psf ground snow load per Howard County Amendment.
 - a. Roof Truss engineering drawings have been attached showing snow load of 40psf.
-

Attention: Steve Rolls - Zoning Review

Comment:

1. *Accessory structure size limit is 2,200 sf. Proposed structure is 4,930 sf. Reduction in size or variance required.*
 - a. Our variance was denied so we have redesigned to the structure to be within the 2,200-sf size limit.
-

Attention: Health Department Review

Comment:

1. *Need a plot plan showing existing septic and how new toilet will be connected. - ZS*
 - a. We have removed the toilet.
-

Please let me know if you have any questions or need anything else. Thank you.

Sincerely,



Dustin L Watson AIA, LEED AP BD+C, CDP
Founding Principal





inPLACE Design LLC
2002 Clipper Park Road
Suite 110A
Baltimore, MD 21211 USA

Date: August 22, 2023

To: Howard County
Inspections, Licenses & Permits
3430 Courthouse Drive
Ellicott City, MD 21043

Attn: Plan Reviewers
RE: Health Department

Permit No: B23000244
Project Name: Brooke – Residential Accessory Structure

Attention: Health Department Reviewer

Comment:

1. *Need to show the septic drain fields on the plot plan. There is a "septic as-built drawing from 2019" that needs to be referenced and transposed onto the current large-scaled plot plan. It appears that the drain fields go beyond the sewage disposal area. - ZS*
 - a. We have referenced and transposed the existing septic drain fields based on the as-built drawing onto the large scale site plan.

Please let me know if you have any questions or need anything else. Thank you.

Sincerely,

A handwritten signature in black ink that reads "Dustin L. Watson".

Dustin L Watson AIA, LEED AP BD+C, CDP
Founding Principal



PROFESSIONAL CERTIFICATION
 I CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED
 BY ME, AND THAT I AM A DULY LICENSED ARCHITECT UNDER THE LAWS OF
 THE STATE OF MARYLAND.
 LICENSE NUMBER: 13895
 EXPIRATION DATE: 06/15/2024

ISSUES:

NO.	DESCRIPTION	DATE
1	PERMIT DRAWING SET	2023/01/03

REVISIONS:

NO.	DESCRIPTION	DATE
1	REVISION 1	2023/04/04
2	REVISION 2	2023/08/07

PROJECT NAME:

ICE RINK STRUCTURE

PROJECT NUMBER: 220022 DATE: 2023/08/07

CHECKED BY: DW SCALE: AS NOTED

SHEET NAME:

A 0.1

SHEET NUMBER:

SITE PLANS

GENERAL NOTES:

1. ALL CONSTRUCTION SHALL BE IN COMPLIANCE WITH THE FOLLOWING CODE: IRC 2015; AND IN ACCORDANCE WITH ALL LOCAL AND STATE CODES ORDINANCES AND REGULATIONS
2. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AFFECTING THEIR WORK WITH ACTUAL CONDITIONS AT THE JOB SITE. IF THERE ARE ANY DISCREPANCIES FROM WHAT IS SHOWN ON THE CONSTRUCTION PLAN, HE MUST IMMEDIATELY REPORT SAME TO THE ENGINEER BEFORE DOING ANY WORK, OTHERWISE THE CONTRACTOR ASSUMES FULL RESPONSIBILITIES.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO THE STREETS OR ROADWAYS AND ASSOCIATED STRUCTURES AND SHALL MAKE REPAIRS AS NECESSARY.
4. THE CONTRACTOR MUST COMPLY WITH ALL FEDERAL, STATE AND LOCAL CODES.
5. ALL WORK AND MATERIALS WHICH DO NOT CONFORM TO THE SPECIFICATIONS ARE SUBJECT TO REMOVAL AND REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
6. DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS.
7. ALL DISTURBED AREAS SHALL BE RESTORED BY THE CONTRACTOR TO THE ORIGINAL CONDITION.
8. NOTIFY OWNER 48 HOURS IN ADVANCE OF ANY UTILITY SHUTDOWN.
9. THE CONTRACTOR SHALL COORDINATE WITH THE OWNER/ARCHITECT ALL ITEMS DESIGNATED TO BE REMOVED OR RELOCATED.
10. THE CONTRACTOR WILL PROTECT ALL UTILITIES DESIGNATED TO REMAIN. ANY DAMAGE BY THE CONTRACTOR TO UTILITIES, ALLEYS, STREETS OR ADJACENT PROPERTIES WILL BE REPLACED/REPAIRED AT THE CONTRACTOR'S EXPENSE.

DRAWING INDEX

SHEET NO.	SHEET TITLE	DATE		SCALE
		DD	REV.	
A 0.1	SITEPLAN & DRAWING INDEX	08/2023	2	AS NOTED
A 1.1	FLOOR PLAN AND ROOF PLAN	08/2023	2	1/4" = 1'-0"
A 1.2	REFLECTED CEILING PLAN	08/2023	2	1/4" = 1'-0"
A 2.1	ELEVATIONS	08/2023	2	1/4" = 1'-0"
A 3.1	SECTIONS	08/2023	2	1/4" = 1'-0"
A 4.1	DETAILS	08/2023	2	1/2" = 1'-0"

CODE ANALYSIS 2015 IRC
 12419 ALL DAUGHTERS LANE
 PARCEL A HIGHLAND, HOWARD COUNTY MD 20777

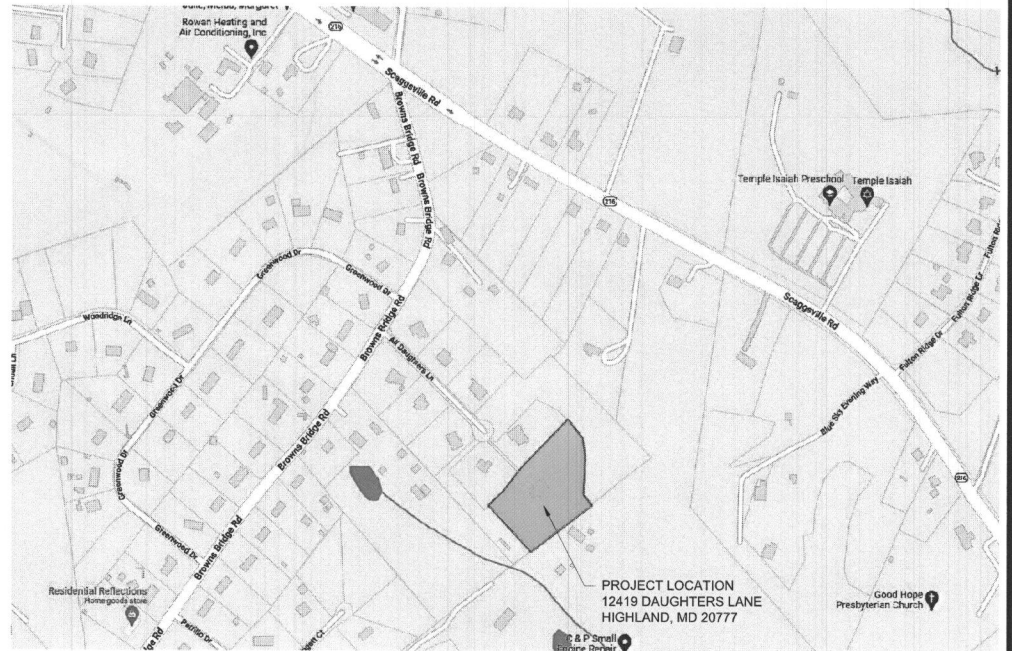
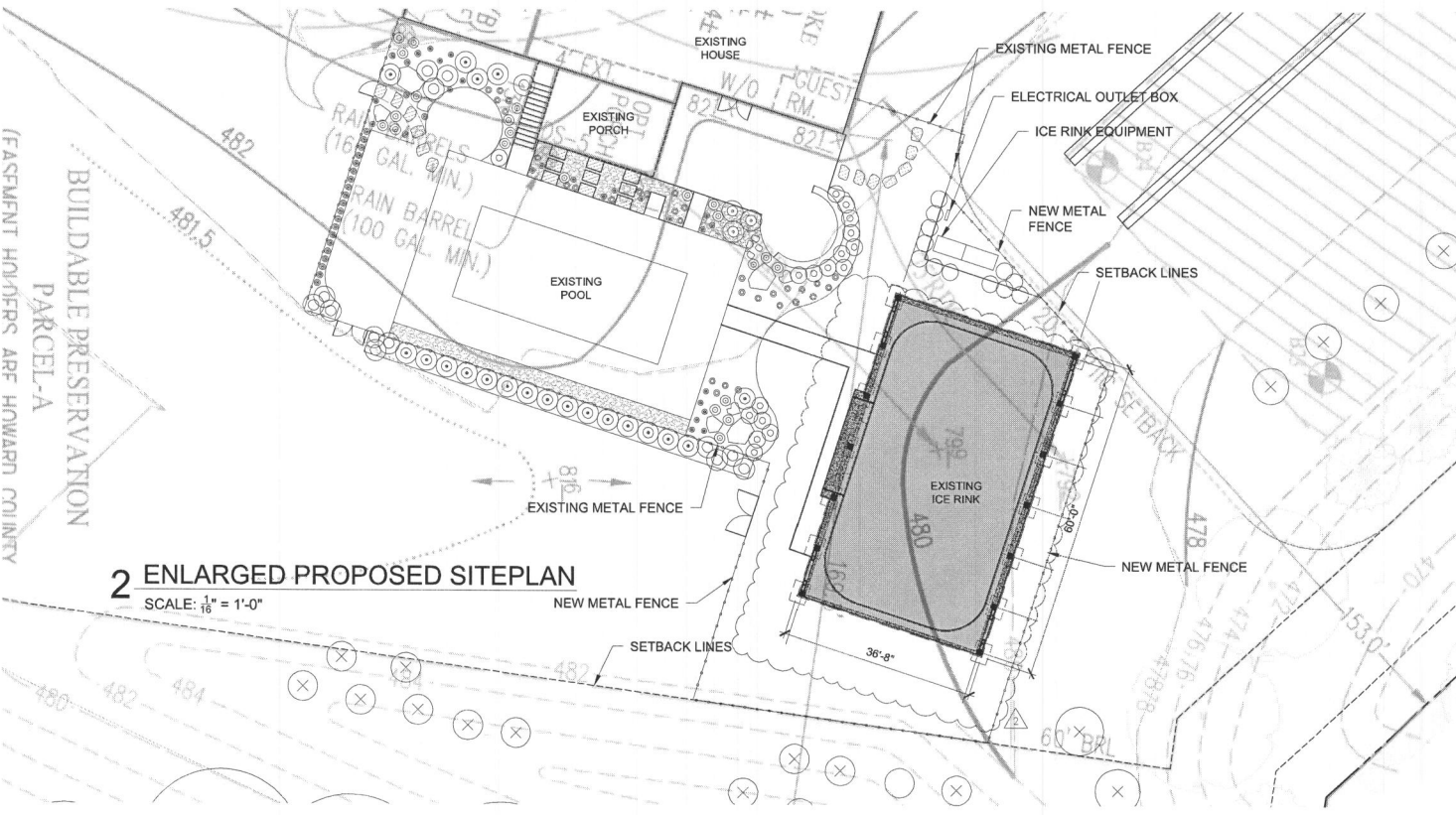
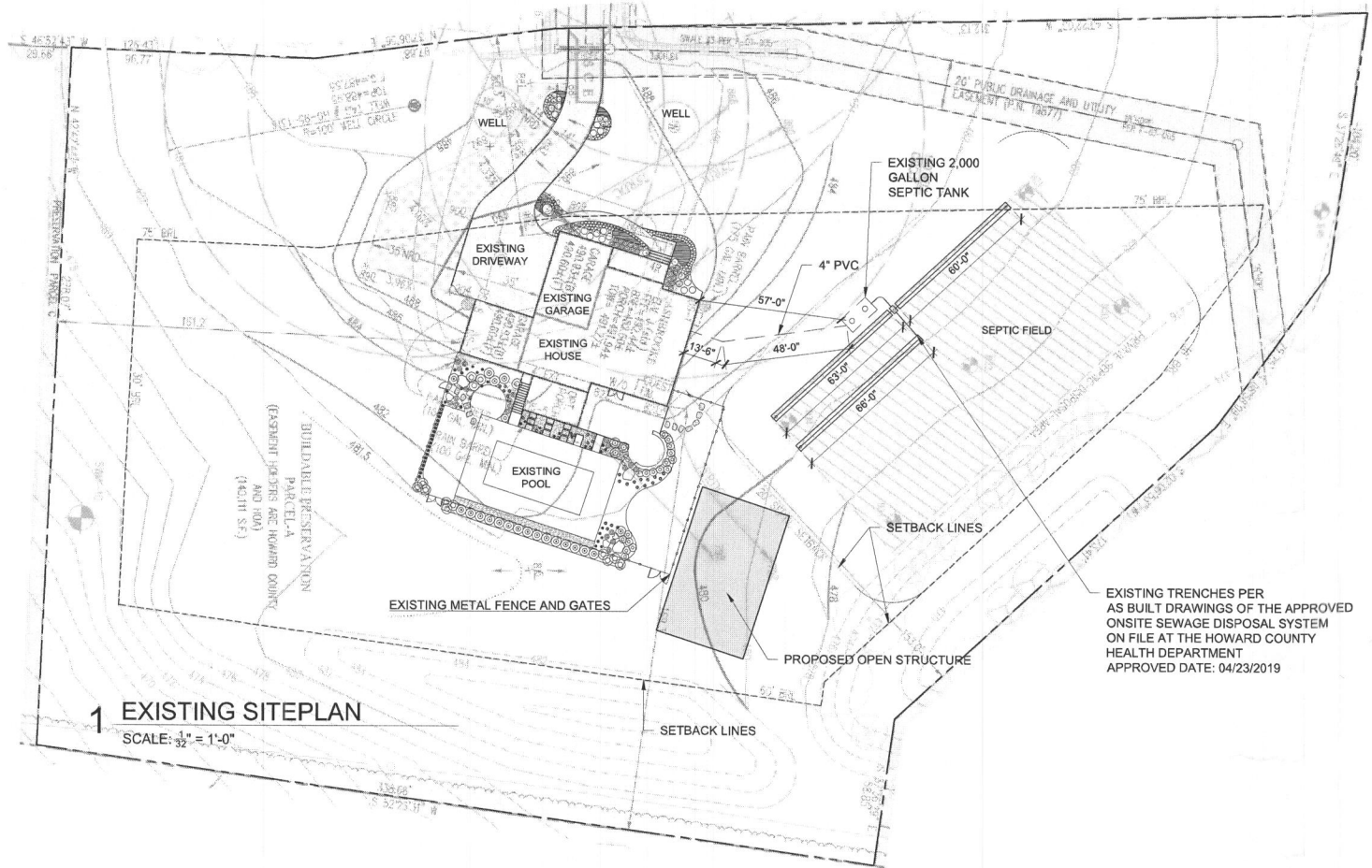
LOT AREA 140,111 S.F.
 EXISTING ZONING: RR-DEO
 PROPOSED STRUCTURE: 2,200 S.F.
 STRUCTURE HEIGHT 24'-6"

DESIGN PARAMETERS

WIND SPEED : 115 MPH
 FROST LINE DEPTH : 30"
 TERMITE : MODERATE TO HEAVY
 WINTER TEMP : 13 DF

PROJECT DESCRIPTION

THE WORK PROPOSED IS FOR AN ACCESSORY STRUCTURE TO COVER AN EXISTING ICE RINK AND WILL BE OPEN ON FOUR SIDES. THE EXTERIOR IS TO MATCH THE EXISTING HOUSE.



3 LOCATION PLAN
 SCALE: N.T.S.

12419 ALL DAUGHTERS LANE
 HIGHLAND, HOWARD COUNTY MD 20777
 LOT AREA: 140,111 S.F. (3.21 AC)
 EXISTING ZONING: RR-DEO
 TAX MAP 40, GRID 18, PARCEL 0178
 5TH ELECTION DISTRICT
 SUBDIVISION: ORCHARD AT BROWN'S BRIDGE
 PLAT NUMBER: 19877

883' FROM PROPERTY TO THE INTERSECTION OF
 DAUGHTERS LANE AND BROWNS BRIDGE ROAD
 DRIVEWAY TO ALL DAUGHTERS LANE
 209' FROM PROPERTY TO ALL DAUGHTERS LANE

Architect/Designer
 inPLACE Design
 2002 Clipper Park Road, Suite 110A
 Baltimore, MD 21211 USA
 Tel | 410 235 0100
 info@inplace-design.com

MEP

Structural Engineer

ORNDORF & ASSOCIATES
 8600 West Chester Pike, Suite 201
 Upper Darby, PA 19082
 Tel | (610) 896-4500
 www.orndorf.com

Client/Owner

JASON BROOKE
 12419 All Daughters Lane
 Highland, MD 20777

SEAL/SIGN



PROFESSIONAL CERTIFICATION
 I CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED
 BY ME AND THAT I AM A DULY LICENSED ARCHITECT UNDER THE LAWS OF
 THE STATE OF MARYLAND.
 LICENSE NUMBER 13867
 EXPIRATION DATE 08/07/2023

ISSUES:

NO.	DESCRIPTION	DATE
1	PERMIT DRAWING SET	2023/01/03

REVISIONS:

NO.	DESCRIPTION	DATE
1	PETITION REVISIONS	05/03/2022
2	REVISIONS	08/07/2023

PROJECT NAME:

ICE RINK STRUCTURE

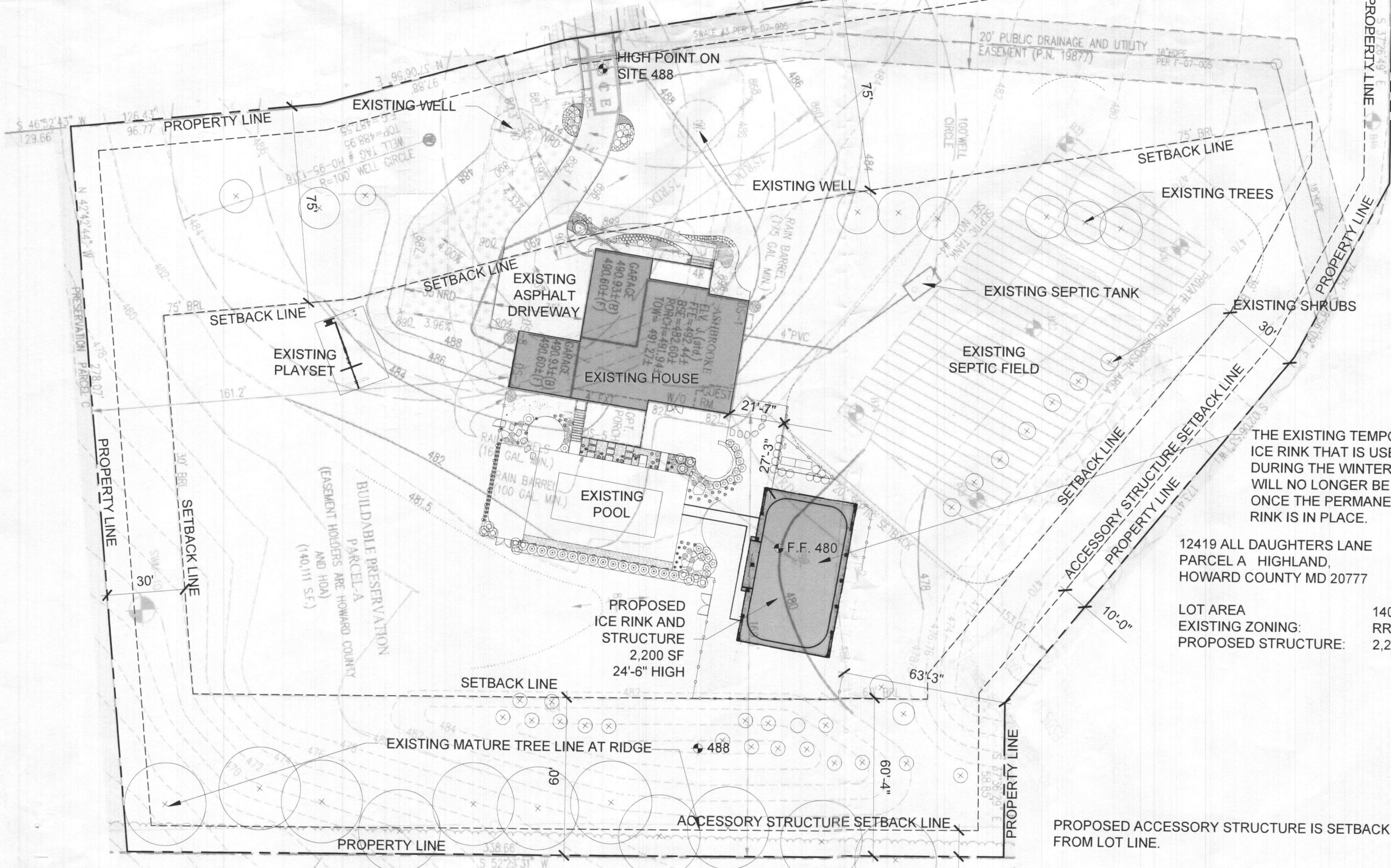
PROJECT NUMBER: 220022 DATE: 2023/08/07
 CHECKED BY: DW SCALE: 1"=40'

SHEET NAME:

PLOT PLAN

SHEET NUMBER:

0.0



THE EXISTING TEMPORARY
 ICE RINK THAT IS USED
 DURING THE WINTER SEASON
 WILL NO LONGER BE USED
 ONCE THE PERMANENT ICE
 RINK IS IN PLACE.

12419 ALL DAUGHTERS LANE
 PARCEL A HIGHLAND,
 HOWARD COUNTY MD 20777

LOT AREA 140,111 S.F.
 EXISTING ZONING: RR-DEO
 PROPOSED STRUCTURE: 2,200 SF

PROPOSED ACCESSORY STRUCTURE IS SETBACK 60'-4"
 FROM LOT LINE.

PROPOSED STRUCTURE: 2,200 SF
 THE RINK WILL BE A CONCRETE SLAB WITH FRAMED STRUCTURE AND ASPHALT SHINGLE ROOF AND SIDING TO MATCH EXISTING
 HOUSE STRUCTURE. THE STRUCTURE WILL BE OPEN ON 4 SIDES.

