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 10701 Hunting Lane
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Seal/Signature



PROFESSIONAL CERTIFICATION: I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED BY ME, AND THAT I AM A DULY LICENSED ARCHITECT UNDER THE LAWS OF THE STATE OF MARYLAND
 REGISTRATION #: 17-908 EXPIRATION: 12/18/2022

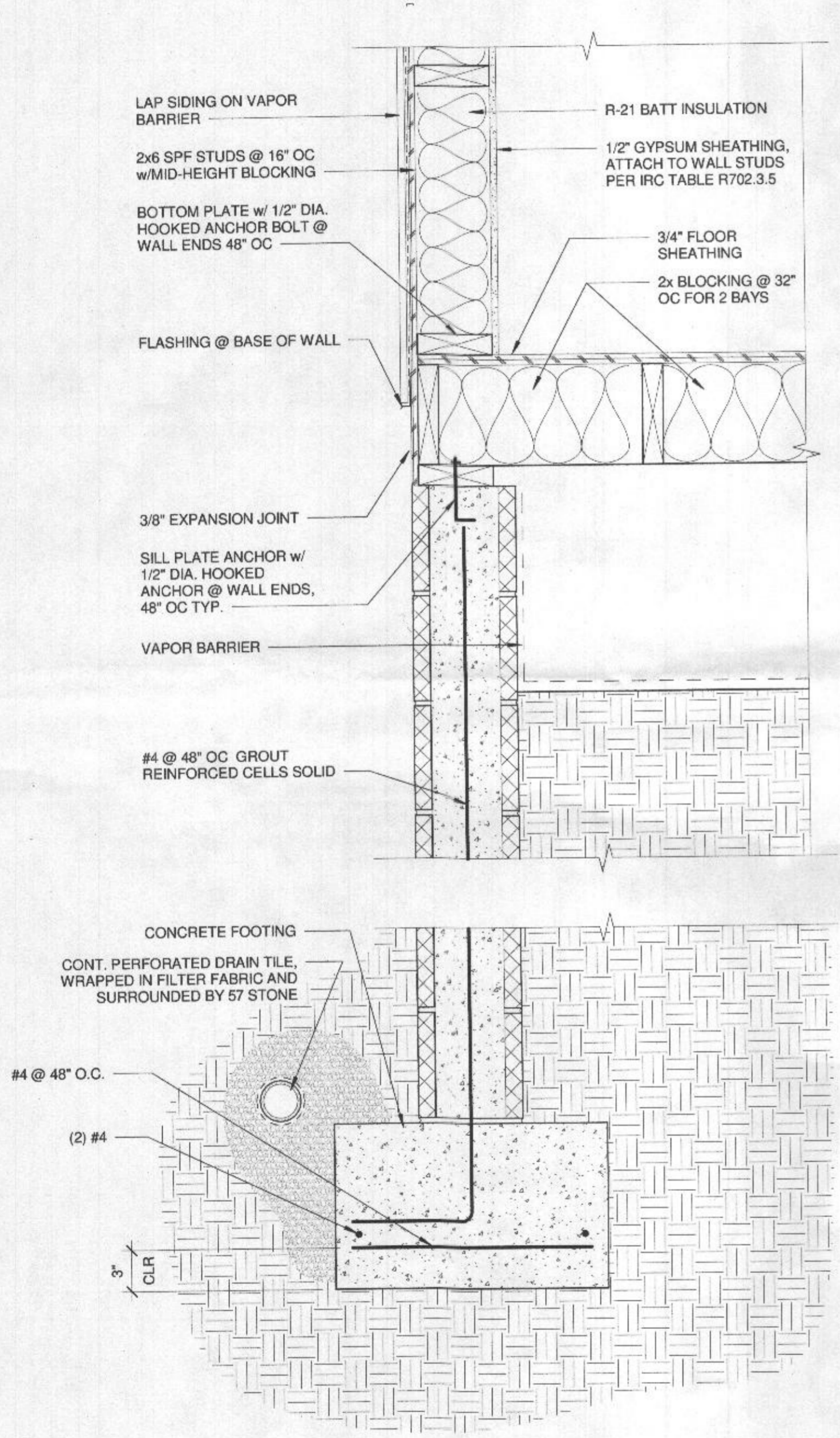
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No.	Date	Title
	05/17/21	PERMIT SET

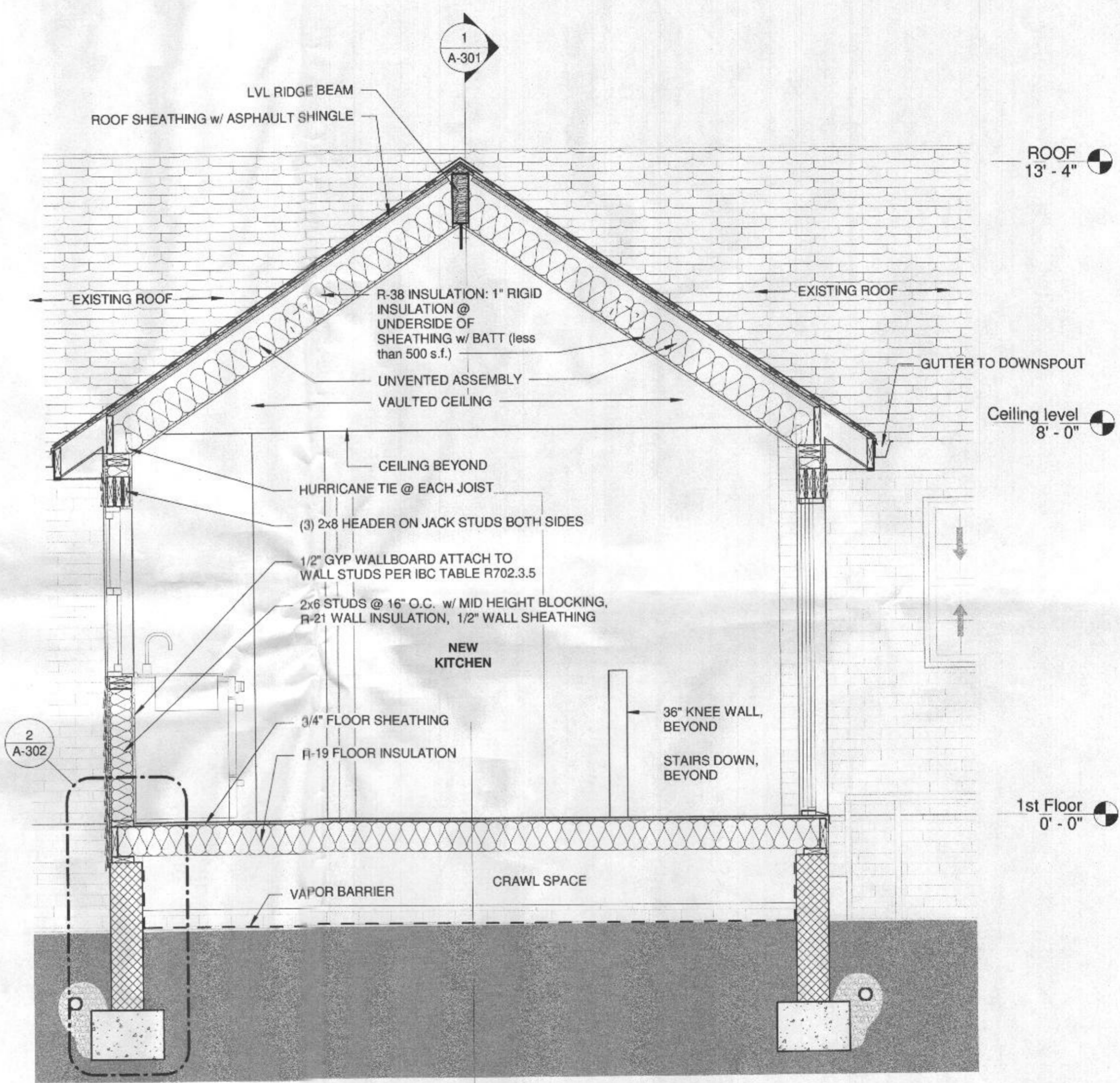
BUILDING SECTIONS

Issue Date: 05/17/2021

A-302



2 SECTION DETAIL
 1 1/2" = 1'-0"



1 BUILDING SECTION
 1/2" = 1'-0"

ROOF
 13' - 4"

Ceiling level
 8' - 0"

1st Floor
 0' - 0"



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 ARCHITECT
 17908
 AMIR M. EZZAT
 STATE OF MARYLAND
 5/17/2021

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GENERAL NOTES

Issue Date: 05/17/2021

S-001

PART 6 - MASONRY

6.1 CODES:
 A. BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES, ACI 530 / ASCE 5 / TMS 402-05.
 B. SPECIFICATIONS FOR MASONRY STRUCTURES, ACI 530.1 / ASCE 6 / TMS 402-09.

6.2 MATERIALS:
 A. LOAD BEARING CONCRETE MASONRY:
 1. HOLLOW AND SOLID
 ASTM C90, NORMAL WEIGHT, NET AREA COMPRESSIVE STRENGTH OF CONCRETE MASONRY UNITS = 1900 PSI. ALL UNITS TO BE TWO CORE TYPE.
 2. BRICK
 ASTM C216 (CLAY OR SHALE), MINIMUM COMPRESSIVE STRENGTH ON NET AREA = 2000 PSI
 ASTM C270 - TYPE S. PROPORTIONS MIX BY VOLUME 1: 1 1/2: 3
 ASTM C476, MINIMUM COMPRESSIVE STRENGTH ON NET AREA = 2000 PSI
 ASTM C444, FINENESS MODULUS 2.0 TO 2.5
 ASTM C207, TYPE S
 ASTM C150, TYPE I OR II
 ASTM A815, GRADE 60

B. FACE BRICK
 ASTM C216 (CLAY OR SHALE), MINIMUM COMPRESSIVE STRENGTH ON NET AREA = 2000 PSI
 ASTM C270 - TYPE S. PROPORTIONS MIX BY VOLUME 1: 1 1/2: 3
 ASTM C476, MINIMUM COMPRESSIVE STRENGTH ON NET AREA = 2000 PSI
 ASTM C444, FINENESS MODULUS 2.0 TO 2.5
 ASTM C207, TYPE S
 ASTM C150, TYPE I OR II
 ASTM A815, GRADE 60

C. MORTAR
 D. GROUT

E. HORIZONTAL JOINT REINF.
 F. COMP. STRENGTH OF M.A.S.

G. WATER
 H. SAND
 I. HYDRATED LIME
 J. PORTLAND CEMENT
 K. REINFORCING BARS
 ASTM A615, GRADE 60

6.3 GENERAL:
 A. PROVIDE GALVANIZED STANDARD WEIGHT HORIZONTAL JOINT REINFORCEMENT IN 8 IN. WALLS AND PARTITIONS AT 16 IN. O.C. PROVIDE ONE PIECE PREFABRICATED UNITS AT 8 IN. O.C. AT ALL WALL CORNERS AND INTERSECTIONS. LAP REINFORCEMENT A MINIMUM OF TWO CELLS.
 B. PROVIDE MASONRY ANCHORS AT 16 IN. O.C. SET ON COURSE AND ATTACHED TO ALL BEAMS, COLUMNS, PARTITIONS, AND WALLS ABUTTING OR ADJACENT TO MASONRY.
 C. PROVIDE CONTINUOUS BOND BEAMS WITH 2#4 HORIZONTAL REINFORCEMENT IN ALL MASONRY WALLS AT EACH FRAMING LEVEL.
 D. ALL PIERS AND PARTITIONS SHALL BE BONDED OR ANCHORED TO ADJACENT MASONRY WALLS. PROVIDE TIES TO ADJACENT FLOOR AND ROOF CONSTRUCTION IN ACCORDANCE WITH DETAILS AND DRAWINGS.
 E. LAP SPICES FOR DEFORMED REINFORCING BARS USED IN MASONRY CONSTRUCTION SHALL BE FULLY GROUTED OR OF 100% SOLID MASONRY UNITS. IN MULTIPLE WYTHE WALLS (CAVITY AND COMPOSITE WALLS) BOND THE WYTHES TOGETHER WITH RIGID METAL TIES OR PREFABRICATED JOINT REINFORCEMENT CONFORMING TO ACI 530 / ASCE 5 REQUIREMENTS. COMPLETELY FILL ALL COLLAR JOINTS IN COMPOSITE WALLS WITH MORTAR OR GROUT.
 F. IN GROUTED AND/OR REINFORCED MASONRY WALLS, USE MASONRY UNITS WITH CORES THAT ALIGN VERTICALLY TO PROVIDE CONTINUOUS UNBROKEN CELLS FOR GROUTING AND REINFORCING STEEL PLACEMENT.
 G. LAP SPICES FOR DEFORMED REINFORCING BARS USED IN MASONRY CONSTRUCTION SHALL BE 30 BAR DIAMETERS.
 H. ALL WALL SECTIONS AND PIERS LESS THAN 4 FEET SQUARE IN CROSS-SECTIONAL AREA TO BE FULLY GROUTED OR OF 100% SOLID MASONRY UNITS.
 I. CONTRACTOR SHALL PROVIDE ADEQUATE BRACING AND SUPPORT OF ALL MASONRY WORK UNTIL PERMANENT CONSTRUCTION IS IN PLACE.
 J. SEE SPECIFICATIONS AND DETAILS FOR GENERAL CONTROL JOINT REQUIREMENTS. JOINTS ARE TO BE CONSTRUCTED IN ALL WALLS AND PARTITIONS.
 K. THE CONTRACTOR SHALL VERIFY ALL OPENINGS BELOW LINTELS INDICATED ARE ADEQUATE TO ACCEPT DOOR FRAMES, LOUVERS, WINDOW FRAMES, ETC. AS SHOWN ON THE ARCHITECTURAL AND MECHANICAL DRAWINGS. NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER OF ANY DISCREPANCIES PRIOR TO LINTEL INSTALLATION.
 L. NO OPENINGS SHALL BE PLACED ABOVE ANY LINTEL WITHIN A HEIGHT LESS THAN OR EQUAL TO THE WIDTH OF THE CLEAR OPENING BELOW THE LINTEL, UNLESS DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER.
 M. SUBMIT SHOP DRAWINGS FOR DESIGNER'S APPROVAL, INCLUDING COMPLETE BENDING AND PLACING DETAILS OF ALL REINFORCING STEEL INDICATING POSITION OF SPLICES AND LOCATION OF REINFORCEMENT AND GROUT IN THE MASONRY.
 N. GROUT ALL REINFORCED VERTICAL CORES AND ALL UN-REINFORCED CORES INDICATED TO RECEIVE GROUTING ON THE DRAWINGS. GROUT ALL HORIZONTAL COURSES (BOND BEAMS) WITH HORIZONTAL REINFORCING BARS.

4.3 CONCRETE MIX PROPERTIES:

A. ELEMENT (NORMAL WEIGHT UNO)

1. FOOTINGS:	28-DAY STRENGTH	W/C (MAX)	AIR CONTENT (M)
	3,000 PSI	0.55	6% ± 1.5

B. PORTLAND CEMENT: ASTM C150, TYPE II
 C. CEMENT SUBSTITUTES: ASTM C595, TYPE LS (LIMIT TO 50% MAX OF CEMENTITIOUS CONTENT BY WEIGHT)
 D. AGGREGATES / DENSITY: ASTM C33 / 145 PCF - NORMAL WEIGHT
 E. AIR-ENTRAINMENT: ASTM C260

4.4 STEEL REINFORCEMENT:

1. DEFORMED REINFORCING BARS: ASTM A615 GRADE 60
 2. WELDABLE DEFORMED REINF. BARS: ASTM A706 OR APPROVED EQUAL.
 3. WELDED WIRE REINFORCEMENT (WWR): ASTM A497 OR A185 (FLAT SHEETS ONLY)

4.5 CONCRETE COVER:

A. MILD REINFORCED CONCRETE
 1. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3 IN.
 2. CONCRETE EXPOSED TO EARTH OR WEATHER:
 #6 BAR OR LARGER: 2 IN.
 #5 BAR OR SMALLER: 1 1/2 IN.
 3. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
 SLABS, WALLS AND JOISTS: #11 BAR OR SMALLER: 3/4 IN.
 BEAMS AND COLUMNS: TO TIES, STIRRUPS, OR SPIRALS: 1 1/2 IN.

4.6 GENERAL REQUIREMENTS:

A. REINFORCEMENT AT OPENINGS: UNLESS DETAILED OTHERWISE, PROVIDE 2 #6 AT EACH SIDE OF ALL OPENINGS IN WALLS AND SLABS AND EXTEND 2 FT-6 IN. BEYOND THE OPENING OR AS DETAILED, EXCEPT VERTICAL BARS AT SIDES OF OPENINGS IN WALLS ARE TO EXTEND FROM FLOOR TO FLOOR. BARS MAY BE MOVED ASIDE AT OPENINGS OR SLEEVES, BUT DO NOT CUT OR OMIT.
 B. MINIMUM REINFORCEMENT: REINFORCE ALL WALLS WITH AT LEAST #4 @ 12 IN. EACH WAY EACH FACE AND 2 #6 EACH EDGE, UNLESS DETAILED OTHERWISE.
 C. EXISTING SURFACE TREATMENT: ROUGHEN ALL EXISTING CONCRETE SURFACES COMMON WITH NEW CONCRETE TO AMPLITUDE OF 1/4 INCH.

4.7 SPLICING AND PLACEMENT OF REINFORCEMENT:

A. NO SPLICES OF REINFORCEMENT SHALL BE PERMITTED EXCEPT AS DETAILED OR AUTHORIZED BY THE STRUCTURAL ENGINEER. MAKE BARS CONTINUOUS AROUND CORNERS. WHEN PERMITTED, SPLICES SHALL BE MADE BY CONTACT TENSION LAP SPICE, UNLESS NOTED OTHERWISE.
 B. SPLICE WELDED WIRE REINFORCEMENT TWO FULL MESH LENGTHS AND WIRE TOGETHER.
 C. SPLICE BARS AS SHOWN ON DRAWINGS BUT NOT LESS THAN 50 BAR DIAMETERS FOR SLABS AND BEAM BOTTOM BARS, AND NOT LESS THAN 65 BAR DIAMETERS FOR WALLS AND BEAM TOP STEEL.
 D. NO WELDING OF REINFORCING SHALL BE PERMITTED UNLESS SPECIFICALLY CALLED FOR OR APPROVED BY THE STRUCTURAL ENGINEER.
 E. PROVIDE #4 CHAIR BARS, HIGH CHAIRS, TIES, CLIPS, SLAB BOLSTERS AND OTHER ACCESSORIES WHERE NOT SPECIFIED ON THE DRAWINGS IN ACCORDANCE WITH MANUAL OF STANDARD PRACTICE OR DETAILING REINFORCING CONCRETE STRUCTURES ACI 315 OR (RSI-WRS) MANUAL OF STANDARD PRACTICE. USE PLASTIC TIPS ON ALL CHAIRS PLACED ON THE SIDES OF CONCRETE FORMWORK.
 F. PROVIDE PLASTIC TIPPED BOLSTERS AND CHAIRS AT ALL LOCATIONS WHERE THE CONCRETE SURFACE IS IN CONTACT WITH THE BOLSTERS OR CHAIRS IS EXPOSED.

PART 5 - WOOD FRAMING

5.1 CODES:
 A. "DESIGN SPECIFICATIONS", TIMBER CONSTRUCTION MANUAL, AMERICAN INSTITUTE OF WOOD CONSTRUCTION.
 B. "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" WITH SUPPLEMENT, 2012, AMERICAN FOREST & PAPER ASSOCIATION.
 C. "PERFORMANCE STANDARD AND POLICIES FOR STRUCTURAL USE PANELS", PRP-108, AMERICAN PLYWOOD ASSOCIATION.

5.2 MATERIALS:
 A. SAWN LUMBER: ALL SAWN LUMBER SHALL HAVE 19% MAXIMUM MOISTURE CONTENT AND SHALL BE SURFACE DRY.
 1. SPRUCE-PINE-FIR (NOT SOUTH):

STRUCTURAL MEMBERS: NO. 1 / NO. 2 OR BETTER WITH THE FOLLOWING BASE DESIGN VALUES:	FB	FT	FV	FC(PERP)	EC(PARA)	E
	450	450	175	425	1,150	1,400,000

 2. SOUTHERN PINE (PRESSURE TREATED): GRADE NO.2 OR BETTER WITH THE BASE DESIGN VALUES LISTED IN TABLE 4B OF THE NDS.
 B. MANUFACTURED WOOD BEAMS: UTILITY FRAMING MEMBERS SHALL BE "MICROLAM" LVL OR APPROVED EQUAL WITH THE FOLLOWING MECHANICAL PROPERTIES AND MINIMUM STRENGTH VALUES (PSI):

FB	FT	FV	FC(PERP)	EC(PARA)	E
2,400	1555	285	750	2,510	3,000,000

 C. MANUFACTURED WOOD COLUMNS: UTILITY FRAMING MEMBERS SHALL BE "1.8E PARALAM PSL" OR APPROVED EQUAL WITH THE FOLLOWING MECHANICAL PROPERTIES AND MINIMUM DESIGN STRENGTH VALUES (PSI):

FB	FV	FC(PERP)	EC(PARA)	E
2,400	190	425	2,500	1,800,000

 5.3 SAWN LUMBER:
 A. SEE IRC TABLE 6A02.3 FOR MINIMUM BRACING AND FASTENING.
 B. MEMBERS SHALL BE SET WITH CROWN SIDE UP AND HAVE A MINIMUM 3 IN. BEARING.
 C. MEMBERS FRAMING TO BEAMS, HEADERS, ETC. SHALL BE SECURED WITH SIMPSON STRONG-TIE FRAMING ANCHORS OR APPROVED EQUAL, UNLESS DETAILED OTHERWISE.
 D. ALL JOISTS AND RAFTERS SHALL BE RIGIDLY BRIDGED AT INTERVALS NOT EXCEEDING 8 FEET.
 E. USE 1/2 IN. DIAMETER LAG SCREWS OR THRU BOLTS AT 24 IN. O.C. TO JOIN MULTIPLE 2X BEAMS OR GIRDERS SO THAT LOAD DISTRIBUTES EQUALLY.
 F. PROVIDE CONTINUOUS SOLID BLOCKING UNDER CONCENTRATED LOADS (INCLUDING WINDOW AND DOOR JAMBS) DOWN THROUGH FLOOR FRAMING TO SLAB ON GRADE OR FOUNDATION.
 G. ALL WOOD SILL PLATES SHALL BE ANCHORED TO GROUT FILLED CMU OR CONCRETE FOUNDATIONS WITH 1/2 IN. DIAMETER HOOKED ANCHORS AT 4 FEET O.C. (2 MINIMUM PER SILL PIECE). ANCHOR BOLTS SHALL BE EMBEDDED A MINIMUM OF 14 IN. INTO MORTAR GROUT AND 8 IN. INTO CAST-IN-PLACE CONCRETE.
 H. ALL BOLTS AND LAG SCREWS SHALL BE FITTED WITH GALVANIZED, MALLEABLE IRON OR STEEL PLATE WASHERS.

5.4 WOOD PRESERVATIVE TREATMENT:
 A. WRESE LUMBER OR PLYWOOD IS INDICATED AS TREATED, COMPLY WITH APPLICABLE REQUIREMENTS OR AMERICAN WOOD PRESERVERS ASSOCIATION (AWPA) STANDARDS U1, T1 AND P5, AND WITH AWPP STANDARDS LISTED BELOW. MARK EACH TREATED ITEM WITH THE AWPP QUALITY MARK REQUIREMENTS.
 B. PRESSURE TREAT ABOVE-GROUND ITEMS WITH WATER-BORNE PRESERVATIVES TO COMPLY WITH AMERICAN WOOD PRESERVATIVES BUREAU (AWPB) LP-2. AFTER TREATMENT, KILN DRY LUMBER AND PLYWOOD TO A MAXIMUM MOISTURE CONTENT OF 10 PERCENT AND 15 PERCENT, RESPECTIVELY.
 C. TREAT INDICATED ITEMS AND WOOD SILLS, SLEEPERS, BLOCKING, AND SIMILAR CONCEALED MEMBERS IN CONTACT WITH MASONRY OR CONCRETE TO WOOD AND WOOD-TO-STEEL CONNECTIONS. CONNECTORS: SIMPSON STRONG-TIE OR APPROVED EQUAL.

5.5 FASTENERS:
 A. NAILS: COMMON WIRE, GALVANIZED, EXCEPT BRIGHT RING SHANKED FOR ATTACHMENT OF SUBFLOOR PLYWOOD TO FLOOR JOISTS.
 B. BOLTS: ASTM A307 FOR ALL WOOD-TO-WOOD AND LIGHT GAGE STEEL-TO-WOOD CONNECTIONS.
 C. SCREWS: ASME B18.8.1 FOR ALL WOOD-TO-WOOD AND WOOD-TO-STEEL CONNECTIONS. CONNECTORS: SIMPSON STRONG-TIE OR APPROVED EQUAL. IF NOT OTHERWISE NOTED ADHERE TO FASTENING SCHEDULES IN TABLE 6A02.3 OF THE IRC.

5.6 SHEATHING
 A. FACTORY-MARK EACH CONSTRUCTION PANEL WITH APA TRADEMARK INDICATING COMPLIANCE WITH GRADE REQUIREMENTS.
 B. INSTALL PANELS WITH FACE GRAIN PERPENDICULAR TO THE SUPPORTING MEMBERS, UNLESS DETAILED OTHERWISE.
 C. FLOOR: STRUCTURAL FLOOR SHALL BE 3/4" STRUCTURAL 1, APA 24/16 RATED PLYWOOD AND SHALL BE GLUED AND NAILED OVER JOISTS WITH 8d NAILS AT 6" ON CENTER.
 D. WALLS: ALL EXTERIOR WOOD STUD WALLS SHALL BE SHEATHED WITH 19/32 STRUCTURAL 1, EXPOSURE 1, SPAN RATING 32/16 WOOD SHEATHING UNLESS NOTED OTHERWISE IN SHEAR WALL SCHEDULE. UNLESS NOTED OTHERWISE, ATTACH WOOD SHEATHING USING 8d NAILS AT 6" ON CENTER AT PANEL EDGES AND 12" ON CENTER FOR THE FIELD. SEE SHEAR WALL SCHEDULE FOR STRUCTURAL PANELS ON INTERIOR WALLS. INTERIOR GYPSUM WALL BOARD SHALL BE SECURED TO ALL TOP AND BOTTOM PLATE, STUDS AND BLOCKING WITH 6 PENNY COOLER NAILS AT 7" ON CENTER MINIMUM OR #6 DRYWALL SCREWS AT 10" ON CENTER MAXIMUM EACH FACE.
 E. ROOF: TYPICAL SHEATHING SHALL BE 19/32 STRUCTURAL 1, APA RATED TONGUE AND GROOVE PLYWOOD WITH A 40/32 SPAN RATING GLUED AND NAILED WITH 8d NAILS AT 6" ON CENTER TO ROOF RAFTERS, JOISTS AND TRUSSES.

GENERAL STRUCTURAL NOTES

PART 1 - GENERAL REQUIREMENTS AND DESIGN CRITERIA

1.1 SPECIFICATIONS
 A. THE WORK OF THESE DRAWINGS ADDRESSES STRUCTURAL INFORMATION ONLY. THE STRUCTURAL DOCUMENTS INCLUDE THESE S-SERIES DRAWINGS AND GENERAL NOTES. THERE ARE NO TECHNICAL SPECIFICATIONS IN ADDITION TO THESE GENERAL NOTES.

1.2 ELEVATIONS & DIMENSIONS
 A. ALL ELEVATIONS AND DIMENSIONS SHOWN FOR NEW CONSTRUCTION ARE BASED ON THE ARCHITECTURAL DRAWINGS. COORDINATE ALL ELEVATIONS AND DIMENSIONS BEFORE PROCEEDING WITH CONSTRUCTION. EXISTING DIMENSIONS ARE TO BE VERIFIED IN THE FIELD BY THE CONTRACTOR, AS APPROPRIATE, PRIOR TO FABRICATION OF MEMBERS.

1.3 GOVERNING BUILDING CODES
 A. THE FOLLOWING BUILDING CODES AND STANDARDS, INCLUDING ALL SPECIFICATIONS REFERENCED WITHIN, SHALL APPLY TO THE DESIGN, CONSTRUCTION, QUALITY CONTROL AND SAFETY OF ALL WORK PERFORMED ON THE PROJECT.
 1. "INTERNATIONAL RESIDENTIAL CODE - 2018", INTERNATIONAL CODE COUNCIL.
 2. "MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES", (ASCE/ASCE 7-16, 2016), AMERICAN SOCIETY OF CIVIL ENGINEERS.
 B. ADDITIONAL CODES FOR MATERIALS SHALL BE FOUND IN THE APPROPRIATE SECTIONS THAT FOLLOW. SEE THOSE SECTIONS FOR THE APPLICABLE CODES.

1.4 DESIGN LOADS
 A. FLOOR LIVE LOADS:
 1. PRIVATE ROOMS/CORRIDORS/BALCONIES: 40 PSF
 B. ROOF LIVE LOADS:
 1. MINIMUM LIVE LOAD: 40 PSF (30 PSF FOR ROOF SLOPE < 2%)
 2. ROOF SHOW LOAD - PLUS DRIFTING AND SLIDING WHERE APPLICABLE
 a. PG = 30 PSF
 b. CE = 1.0
 c. I = 1.0
 d. CT = 1.0

D. DEAD LOADS - ALL PERMANENT STATIONARY CONSTRUCTION
 1. FLOOR DESIGN LOAD = 15 PSF

E. WIND LOAD PARAMETERS
 1. BASIC WIND SPEED (3-SECOND GUST), V = 90 MPH (SERVICE), V = 115 MPH (ULTIMATE)
 2. WIND IMPORTANCE FACTOR, I = 1.0, AND OCCUPANCY CATEGORY = II
 3. EXPOSURE CATEGORY: B

1.5 DESIGN CRITERIA
 A. EXISTING BRACING METHOD - LIGHT FRAMED WOOD WALLS.
 B. RENOVATION SCOPE INCLUDES PARTIAL DEMOLITION OF REAR WALL AND CONSTRUCTION OF NEW 1 STORY WOOD FRAMED ADDITION. RECONSTRUCTION BRACING REQUIREMENTS IRC 602.010.3(1).
 C. MAX BRACE WALL SPACING = 15 FT
 1. BRACING METHOD - CONTINUOUSLY SHEATHED WOOD STRUCTURAL PANELS, WSP.
 2. FIRST FLOOR TO ROOF
 a. NORTH-SOUTH - MINIMUM TOTAL LENGTH OF WSP WOOD STRUCTURAL PANEL REQUIRED = 2.0 FT. MINIMUM TOTAL LENGTH OF WOOD STRUCTURAL PANEL PROVIDED = 6.5 FT.
 b. EAST-WEST - MINIMUM TOTAL LENGTH OF WSP WOOD STRUCTURAL PANEL REQUIRED = 1.5 FT. MINIMUM TOTAL LENGTH OF WOOD STRUCTURAL PANEL PROVIDED = 7.92 FT.
 C. REFER TO IRC FOR INSTALLATION INSTRUCTIONS OF WSP.
 D. AT INTERIOR SIDE OF WSP WALLS ATTACH MINIMUM 1/2" GYPSUM BOARD SHEATHING TO WALLS (SEE ARCH FOR THICKNESS) AND ATTACH TO STUDS PER IRC TABLE R702.3.5

PART 2 - CONSTRUCTION

2.1 GENERAL
 F. THESE DRAWINGS REPRESENT THE COMPLETED PROJECT WHICH HAS BEEN DESIGNED FOR THE WEIGHTS OF MATERIALS, FOR THE SUPERIMPOSED LOADS INDICATED IN THE DESIGN LOAD CRITERIA ABOVE, AND FOR LOADS INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ALLOWABLE CONSTRUCTION LOADS AND TO PROVIDE PROPER DESIGN AND CONSTRUCTION OF FALSE WORK, STAGING, BRACING, SHEETING AND SHORING, ETC.
 G. DEVELOPING AND IMPLEMENTING JOB SITE SAFETY AND CONSTRUCTION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. ALL COSTS OF INVESTIGATION AND REDESIGN, DUE TO THE CONTRACTOR MIS-LOCATION OF STRUCTURAL ELEMENTS OR OTHER LACK OF CONFORMANCE WITH THE PROJECT DOCUMENTS, SHALL BE AT THE CONTRACTOR'S EXPENSE.
 H. CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, DRAWINGS FOR SIZE AND LOCATION OF OPENINGS, SLEEVES, INSERTS AND DEPRESSIONS.
 I. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR DETAILED INFORMATION REGARDING FINISHES, WATERPROOFING, FIREPROOFING, ETC.
 J. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF NON-LOAD BEARING MASONRY AND DRYWALL PARTITIONS.
 K. IN CASE OF CONFLICT BETWEEN THE GENERAL NOTES, AND THE DETAILS THE MOST RECENT SHALL GOVERN.
 L. WORK NOT INCLUDED ON THE DRAWINGS BUT IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING LOCATIONS ELSEWHERE ON THE DRAWINGS SHALL BE REPEATED.

PART 3 - FOUNDATIONS / EARTHWORK / GEOTECHNICAL REPORT

3.1 REFERENCE GEOTECHNICAL REPORT:
 A. FOUNDATIONS HAVE BEEN DESIGNED WITH AN ASSUMED BEARING CAPACITY OF 1500 PSF. THE OWNER HAS NOT PROVIDED A GEOTECHNICAL REPORT FOR THIS PROJECT AND THUS HAS ACCEPTED THE RISK. PRIOR TO PLACING FOUNDATIONS THE CONTRACTOR SHALL HAVE AN EXPERIENCED, QUALIFIED GEOTECHNICAL ENGINEER FIELD VERIFY THE BEARING ELEVATIONS FOR FROST DEPTH AND THE ASSUMED ALLOWABLE BEARING CAPACITY. ALL FOUNDATIONS SHALL BEAR A MINIMUM OF 30 INCHES BELOW ADJACENT EXISTING GRADE. THE CONTRACTOR SHALL COORDINATE THESE REQUIREMENTS WITH ALL UNDERGROUND UTILITIES, TUNNELS, ETC. AND NOTIFY THE ARCHITECT AND STRUCTURAL ENGINEER IN ADVANCE OF ANY CONSTRUCTION TO ALLOW FOR ADJUSTMENTS.

3.2 FOUNDATION DESIGN PARAMETERS
 A. SPREAD FOOTINGS:
 1. BUILDING SPREAD AND STRIP FOOTINGS SHALL BEAR ON UNDISTURBED NATURAL SOILS OR PROPERLY PLACED AND COMPACTED ENGINEERED FILL WITH AN ALLOWABLE BEARING PRESSURE OF 1500 PSF.

3.3 EXCAVATION:
 A. THE SLOPE BETWEEN THE LOWER EDGES OF ADJACENT FOUNDATIONS SHALL NOT EXCEED 30 DEGREES REFERENCED FROM THE HORIZONTAL, UNLESS NOTED OR DETAILED OTHERWISE ON THE PLAN. MAINTAIN A 1V:2H SLOPE FROM BOTTOM EDGE OF ANY EXCAVATION.
 B. THE CONTRACTOR SHALL VERIFY ALL EXISTING FIELD CONDITIONS THAT MAY AFFECT THE INSTALLATION OF THE FOUNDATION SYSTEM AS SHOWN PRIOR TO STARTING WORK.
 C. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL EXISTING UTILITIES, ABOVE AND BELOW GRADE STRUCTURES, ETC., WHETHER INDICATED OR NOT, THAT MAY BE AFFECTED BY THE CONSTRUCTION PROCESS.
 D. UTILITIES LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT THE STRUCTURAL ENGINEERS APPROVAL UNLESS DETAILED OTHERWISE IN THE PLANS.

3.4 FOUNDATION PLACEMENT & PROTECTION:
 A. DO NOT PLACE FOUNDATION CONCRETE IN WATER OR ON FROZEN GROUND. PRETECT IN-PLACE FOUNDATIONS AND SLABS FROM FROST PENETRATION UNTIL THE PROJECT IS COMPLETE. DO NOT USE SALT OR CHLORIDE COMPOUNDS TO DE-ICE THE SITE.
 B. CONCRETE FOR FOUNDATIONS SHALL BE Poured ON THE SAME DAY SUBGRADE APPROVAL IS GIVEN BY THE GEOTECHNICAL ENGINEER.
 C. PRIOR TO PLACING FOUNDATIONS, AN EXPERIENCED, QUALIFIED GEOTECHNICAL ENGINEER SHALL FIELD VERIFY ALLOWABLE BEARING PRESSURES AND DETERMINE FINAL BEARING ELEVATIONS.

PART 4 - CONCRETE WORK

4.1 CODES:
 A. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318-11, AMERICAN CONCRETE INSTITUTE.
 B. ACI MANUAL OF CONCRETE PRACTICE - PARTS 1 THROUGH 5, AMERICAN CONCRETE INSTITUTE.

4.2 STANDARD SPECIFICATIONS AND REFERENCE STANDARDS:
 A. "MANUAL OF STANDARD PRACTICE", CONCRETE REINFORCING STEEL INSTITUTE.
 B. FOLLOW THE LATEST RECOMMENDATIONS AND SPECIFICATIONS OF THE AMERICAN CONCRETE INSTITUTE:
 1. ACI 301 STRUCTURAL CONCRETE FOR BUILDINGS
 2. ACI 302 CONCRETE FLOOR AND SLAB CONSTRUCTION
 3. ACI 304 MEASURING, MIXING, TRANSPORTING AND PLACING CONCRETE
 4. ACI 305 HOT WEATHER CONCRETING
 5. ACI 306 COLD WEATHER CONCRETING
 6. ACI 315 DETAILING REINFORCING STEEL
 7. ACI 318 GENERAL DESIGN OF ITEMS NOT OTHERWISE SPECIFIED
 8. ACI 347 FORMWORK