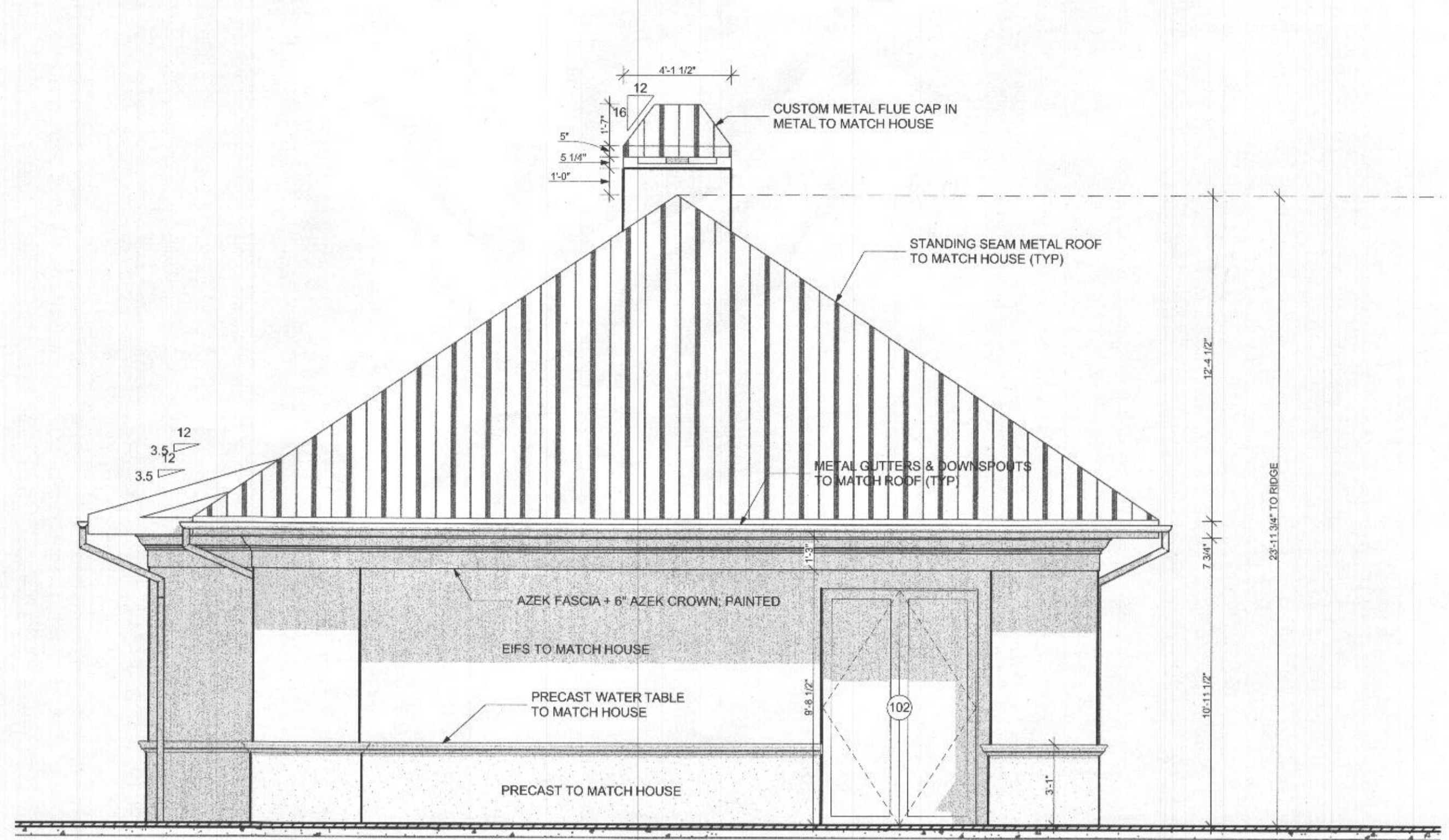




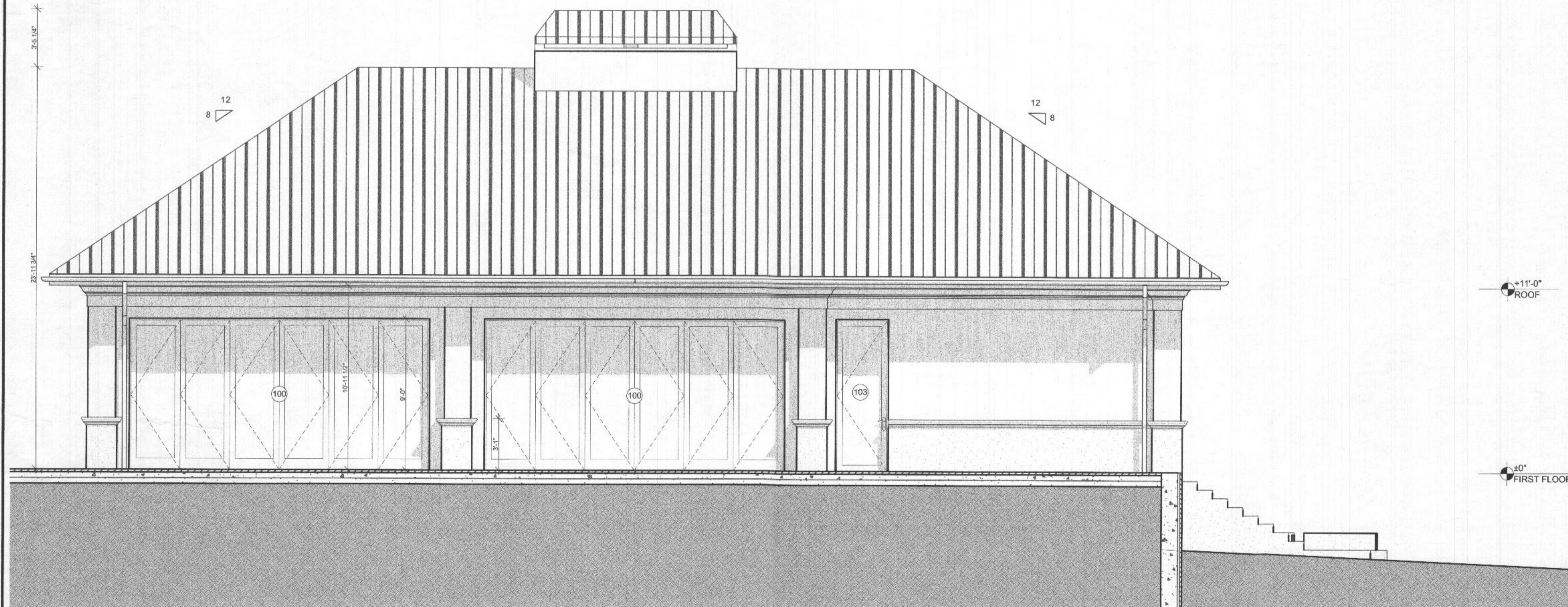
**1 CABANA GARDEN SIDE ELEVATION**  
SCALE: 1/4" = 1'-0"



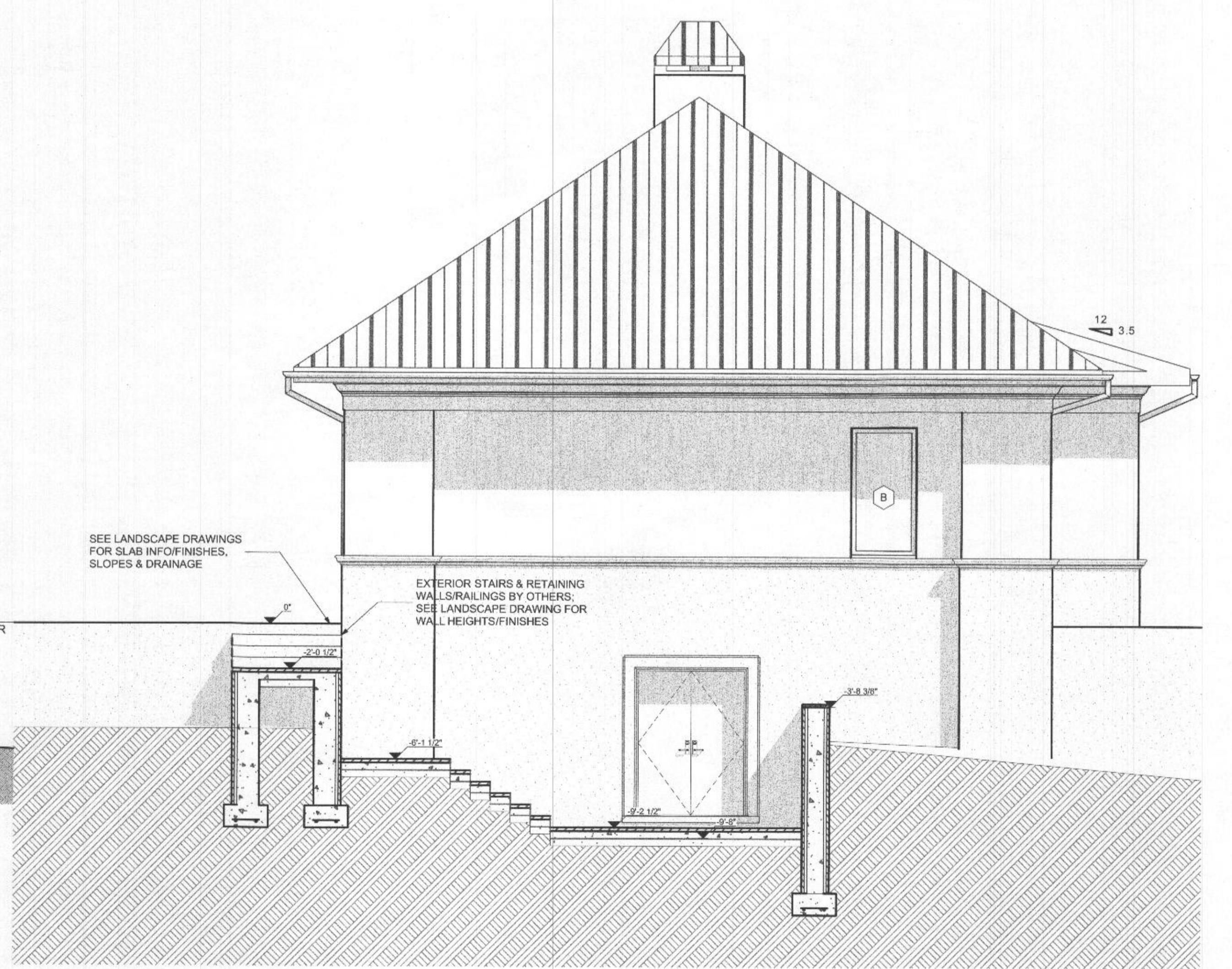
**2 SIDE ELEVATION (1)**  
SCALE: 1/4" = 1'-0"



**3 CABANA POOL SIDE ELEVATION**  
SCALE: 1/4" = 1'-0"



**4 SIDE ELEVATION @ EXT. STAIR**  
SCALE: 1/4" = 1'-0"



EXTERIOR DOOR SCHEDULE							
MARK	SIZE			MATL	QTY	NOTES	VIEW FROM OPENING SIDE (NTS)
	W	HT	THICKNESS				
001	5'-0"	6'-8"	1 3/4"		1	FIBERGLASS EXTERIOR DOOR	
100	16'-0"	9'-0"	1 3/4"		2	MARVIN BIFOLD DOORS; 3L & 3R	
102	6'-0"	9'-0"	1 3/4"		1	MARVIN FIXED INSWING FRENCH DOORS	
103	3'-0"	9'-0"	1 3/4"		2	MARVIN OPERABLE INSWING FRENCH DOOR	
104	16'-0"	9'-0"	1 3/4"		1	MARVIN BIFOLD DOORS; 3L & 3R	

WINDOW SCHEDULE					
MARK	SIZE		QTY	NOTES	VIEW FROM OPENING SI...
	WIDTH	HEIGHT			
A	4'-0"	5'-11 1/8"	3	MARVIN FIXED	
B	3'-0"	5'-11 1/8"	1	MARVIN FIXED	

DATE	REVISIONS

EXTERIOR ELEVATIONS-CABANA  
**THE OVERLOOK CABANA**  
15125 DEVLIN DRIVE  
GLENELG, MARYLAND 21737

DATE: 12/10/2020  
PROJECT NO: 5419



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SHEET NUMBER

**A201**

12/10/2020, 12:27 PM, C:\Users\lbrun\Desktop\PROJECTS\19-0318\19-0318 CABANA REVISED\22220.dwg, © Levin/Brown & Associates, Inc.



**PRE-ENGINEERED WOOD TRUSSES**

TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THESE SPECIFICATIONS AND WHERE ANY APPLICABLE DESIGN FEATURE IS NOT SPECIFIED HEREIN, DESIGN SHALL BE IN ACCORDANCE WITH APPLICABLE PROVISIONS OF LATEST EDITION OF NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION (NDS) AMERICAN FOREST AND PAPER ASSOCIATION (AFPA), AND DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES (ANSI/TPI 1), TRUSS PLATE INSTITUTE (TPI), AND CODES OF JURISDICTION. FABRICATE, SUPPLY AND ERECT WOOD TRUSSES AS SHOWN ON THE DRAWINGS AND AS SPECIFIED. WORK SHALL INCLUDE ALL ANCHORAGE, BLOCKING, CURBING, MISCELLANEOUS FRAMING AND BRACING.

LUMBER USED FOR TRUSS MEMBERS SHALL BE IDENTIFIED BY GRADE MARK OF A LUMBER INSPECTION AGENCY, AND SHALL BE AS SHOWN ON DESIGN DRAWINGS. TRUSSES SHALL BE HANDLED DURING FABRICATION, DELIVERY AND AT JOISTE SO AS NOT TO BE SUBJECTED TO EXCESSIVE BENDING. TRUSSES SHALL BE UNLOADED ON SMOOTH GROUND TO AVOID LATERAL STRAIN. TRUSSES SHALL BE PROTECTED FROM DAMAGE THAT MIGHT RESULT FROM ON-SITE ACTIVITIES AND ENVIRONMENTAL CONDITIONS. PREVENT TOPPLING WHEN BANDING IS REMOVED.

HANDLE DURING INSTALLATION IN ACCORDANCE WITH HANDLING, INSTALLING AND BRACING WOOD TRUSSES (HIB-9), TPI, AND ANSI/TPI 1-1995. INSTALLATION SHALL BE CONSISTENT WITH GOOD WORKMANSHIP AND GOOD BUILDING PRACTICES. TRUSSES SHALL BE SET AND SECURED LEVEL AND PLUMB, AND IN CORRECT LOCATION. TRUSSES SHALL BE HELD IN CORRECT ALIGNMENT UNTIL SPECIFIED PERMANENT BRACING IS INSTALLED. CUTTING AND ALTERING OF TRUSSES IS NOT PERMITTED. CONCENTRATED LOADS (PIL BUNDLES OF DECKING) SHALL NOT BE PLACED ATOP TRUSSES UNTIL ALL SPECIFIED BRACING HAS BEEN INSTALLED AND DECKING IS PERMANENTLY NAILED IN PLACE. ERECTION BRACING IS ALWAYS REQUIRED. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND FURNISHING THE MATERIALS USED FOR INSTALLATION AND PERMANENT BRACING.

STRUCTURAL ENGINEER OF RECORD SHALL APPROVE SHOP DRAWINGS PRIOR TO SUBMITTAL TO BUILDING OFFICIAL. BUILDING OFFICIAL SHALL APPROVE SHOP DRAWING PRIOR TO INSTALLATION. TRUSSES SHALL BE FABRICATED FROM APPROVED SHOP DRAWINGS.

MANUFACTURER SHALL SUBMIT 3 COPIES OF TRUSS DESIGN DRAWINGS BEARING SEAL OF PROFESSIONAL ENGINEER FOR APPROVAL PRIOR TO ERECTION AND ENGINEERING FRAMING PLANS FOR ALL FLAT CHORD TRUSSES. ALL TRUSS SHOP DRAWINGS MUST BE REVIEWED AND APPROVED IN WRITING, BY GENERAL CONTRACTOR, PRIOR TO SUBMITTAL OF SHOP DRAWINGS TO STRUCTURAL ENGINEER AND MUST INCLUDE THE FOLLOWING:

1. STAMP AND SIGNATURE OF ENGINEER, WHO IS REGISTERED IN THE STATE WHERE THE JOB IS TO BE CONSTRUCTED, RESPONSIBLE FOR PREPARATION OF ALL TRUSS DESIGN AND LAYOUT DRAWING.
2. ALLOWABLE LOADS IN LBS/EFFECTIVE NAILED OR PSI FOR LUMBER & PLATES USED AS ALLOWED BY IBCO, CURRENT IBCO REPORT NUMBER & BY SOUTHERN BUILDING CODE CONGRESS INTERNATIONAL.
3. STRESS REDUCTION FACTORS USED FOR PLATES.
4. TOP AND BOTTOM CHORD DESIGN LOADS IN PLF.
5. SIZE, GAUGE, AND EXACT LOCATION BY DIMENSION OF PLATES.
6. LUMBER SPECIES AND GRADES USED.
7. NAME & TRADEMARK OF PLATE MANUFACTURER, TRUSS FABRICATOR & PROJECT NAME/LOCATION & CONCENTRATED LOAD REQUIREMENTS HAVE BEEN DESIGNED FOR AND SHOWN ON DOCUMENTS.
8. TRUSS CONNECTION HARDWARE REQUIREMENTS.

ALL TRUSSES MUST BE DESIGNED FOR UPLIFT LOADS. UPLIFT VALUES @ EACH TRUSS BEARING POINT MUST BE SHOWN ON TRUSS ENGINEERING SHEET.

ALL ROOF TRUSSES SHALL BE ATTACHED TO PERPENDICULAR NON-LOAD BEARING WALLS WITH TRUSS CLIPS. CEILING GMB SHALL BE ATTACHED TO BLOCKING ON THE WALL AND NOT TO THE TRUSS FOR A DISTANCE OF 18" FROM THE WALL.

ALL FLOOR TRUSSES ON THE LOWEST FLOOR 1/4" TRUSSES SHALL BE ATTACHED TO PERPENDICULAR NON-LOAD BEARING WALLS WITH TRUSS CLIPS. CEILING GMB SHALL BE ATTACHED TO BLOCKING ON THE WALL AND NOT TO THE TRUSS FOR A DISTANCE OF 18" FROM THE WALL.

LIVE LOAD DEFLECTION SHALL NOT EXCEED 1/8" OR L/480 FOR FLOOR TRUSSES AND 1/4" OR L/360 FOR ROOF TRUSSES.

THE MANUFACTURER SHALL SUPPLY ALL REQUIRED HANGERS, HOLD-DOWN CLIPS, AND OTHER SPECIAL HARDWARE.

**MASONRY**

ALL MASONRY WORK SHALL CONFORM TO THE APPLICABLE REQUIREMENTS OF BIA AND NCMA SPECIFICATION FOR CONCRETE MASONRY CONSTRUCTION (ACI 531-176) AND SPECIFICATIONS FOR MASONRY STRUCTURE (ACI 530.1-02) PUBLISHED BY THE AMERICAN CONCRETE INSTITUTE.

PROVIDE CONTINUOUS MASONRY BOND BEAM SPANNING ALL EXPANSION JOINTS & WALL INTERSECTIONS.

PROVIDE (2) #5 BENT BARS WITH 3-FOOT LEGS AT EVERY CORNER OR WALL INTERSECTION.

CONTINUOUS TIE OR BOND BEAMS SHALL BE REINFORCED WITH NOT LESS THAN 2 #5 CONTINUOUS BARS. LITELS SHALL BE THE SIZES SHOWN AND REINFORCED AS INDICATED ON THE DRAWINGS.

REINFORCED MASONRY WALLS SHALL HAVE ALL REINFORCED CELLS FILLED WITH CONCRETE. CONCRETE MAY BE PLACED IN MAXIMUM VERTICAL LIFTS NOT TO EXCEED 4-FEET. ROUGHEN ALL SURFACES OF CONCRETE FILL WHICH ARE TO RECEIVE ADDITIONAL LIFTS ABOVE.

MASONRY WALLS SHALL HAVE "DUR-O-WALL" (OR APPROVED EQUIV.) TRUSS TYPE HORIZONTAL REINFORCEMENT AT 16" VERTICALLY ABOVE GRADE AND 8" VERTICALLY BELOW GRADE. COORDINATE BRICK TIE BACK REQUIREMENTS WITH ARCHITECTURAL DRAWINGS. UNLESS NOTED OTHERWISE, STOP ALL HORIZONTAL JOINT REINFORCING AT CONTROL JOINTS.

BRICK VENEER WALLS TO HAVE NON-CORROSIVE METAL TIES AT 16" VERTICALLY AND HORIZONTALLY AND COMPLY WITH ASTM A82 WITH A183, CLASS B-2 COATING. MINIMUM WIRE DIAMETER SHALL BE 0.1875 INCHES. PROVIDE KNEE HOLES AT 24" @ BASE FLASHING.

PROVIDE MIN. 2 COURSES 8x16" SOLID BEARING AT BEAM & HEADER BEARING POINTS IN CMU WALLS.

A36 STEEL LITEL SIZES FOR OPENINGS PER 4" THICKNESS OF MASONRY WALL AS FOLLOWS:  
 4'-0" SPAN OR LESS L3x3 1/2x 5/16" 7'-6" SPAN OR LESS L5x3 1/2x 5/16"  
 5'-6" SPAN OR LESS L4x3 1/2x 5/16" 9'-0" SPAN OR LESS L6x3 1/2x 5/16"  
 PROVIDE MIN. 6" BEARING, EACH END & BRICK TIES, 16" @ 1st COURSE ABOVE LITEL.

FILL SOLIDLY w/2,500psi ASTM C-476 GROUT, ALL BOND BEAMS, CELLS THAT ARE REINFORCED, WILL SECURE EXPANSION BOLTS, SILL PLATE ANCHOR BOLTS OR OTHER MECHANICAL ATTACHMENTS AND ALL CELLS BELOW GRADE.

REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ASTM A-615, GRADE 60. SHOP FABRICATES REINFORCING BARS, WHICH ARE SHOWN TO BE HOOKED, OR BENT. PROVIDE A MINIMUM LAP OF 48 BAR DIAMETERS AT ALL SPLICES, UNLESS INDICATED OTHERWISE.

UNLESS OTHERWISE NOTED, ALL WALLS SHALL BE LAID IN RUNNING BOND. BOND CORNERS AND INTERSECTIONS OF LOAD-BEARING WALLS.

PROVIDE VERTICAL REINFORCING BARS OF THE GIVEN SIZE AND SPACING AS INDICATED. PROVIDE BARS AT ALL WALL CORNERS, INTERSECTIONS AND OPENINGS EDGES.

PROVIDE REBAR DONELS FROM FOUNDATIONS TO MATCH VERTICAL REINFORCING SIZE AND SPACING. DONELS SHALL HAVE STANDARD 90-DEGREE HOOKS AND LAP WITH THE FIRST LIFT OF REINFORCING.

PROVIDE BOND BEAM LITELS AND BRICK SHELF ANGLES ABOVE ALL WALL OPENINGS.

PROVIDE JOIST & BEAM BEARING PLATES w/OTHER ACCESSORIES AS INDICATED, WITH 3 COURSES OF SOLIDLY GROUTED CMU BELOW ALL BEAM BEARINGS OVER A WIDTH OF 2'-8" CENTERED ON THE BEAM.

PROVIDE CMU CONTROL JOINTS AS INDICATED, w/ADDITIONAL JOINTS SUCH THAT THE SPACING BETWEEN JOINTS DOES NOT EXCEED A SPACING OF 3x WALL HEIGHT, 35" MAXIMUM, WHERE BEAMS OR LITELS BEAR AT CMU CONTROL JOINTS, OFFSET 4" LAP THE VERTICAL REINFORCING AS INDICATED.

MASONRY CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY BRACING DURING CONSTRUCTION.

**WOOD FRAMING**

NAIL IN ACCORDANCE WITH RECOMMENDED WOOD FASTENING SCHEDULE IN APPLICABLE BUILDING CODES (LATEST EDITION/HIGH WIND REGION). PROVIDE BLOCKING, BRIDGING AND BRACING PER SAME CODE. AT A MIN., PROVIDE BRIDGING AT EACH END OF THE JOIST, AND ONE ROW OF SOLID BRIDGING BELOW ALL INTERIOR BEARING PARTITIONS.

FASTENERS: JOIST HANGERS, HURRICANE ANCHORS, POST BASES AND OTHER FRAMING ANCHORS ARE TO BE AS MANUFACTURED BY SIMPSON STONE-TIE, U.S.P., OR EQUAL, AND ARE TO BE USED IN STRICT ACCORDANCE WITH MANUFACTURER'S WRITTEN SPECIFICATIONS. ALL FASTENERS TO BE 16 GA. MIN. UNLESS NOTED OTHERWISE. PROVIDE GALV. FINISH UNLESS NOTED OTHERWISE. JOIST HANGERS SHALL BE MIN. 1/4" GA. WITH SIZE AND PROFILE TO SUIT APPLICATION (U.N.O.). PROVIDE JOIST HANGERS FOR ALL FLUSH FRAMED JOISTS. ALL FASTENERS IN CONTACT WITH PRESSURE TREATED WOOD SHALL BE 2-MAX OR TRIPLE ZINC COATED, U.N.O.

THE NUMBER OF WALL STUDS AT BEARING POINTS OF 2X MEMBER BEAMS SHALL EXCEED THE NUMBER OF MEMBERS IN THE BEAM BY ONE. THE CENTERLINE OF THE BEAM SHALL BE THE CENTERLINE OF THE SUPPORTING WALL STUDS. (UNLESS NOTED OTHERWISE ON PLAN) ALL MICRO-LAM BEAMS SHALL HAVE 3 STUDS (MIN. & EXCEED WIDTH OF BEAM). CONTINUE THESE STUDS TO THE FOUNDATION WITH INTERMEDIATE SUPPORTS THROUGH FLOOR, BETWEEN LOWER WALL TOP PLATE & UPPER WALL BOTTOM PLATE.

ALL EXTERIOR POSTS TO BE TREATED 6x6 (U.N.O.). NOTCH TOP OF POST FOR BEAM BRG. (3" MAX.) AND THRU BOLT BEAM TO POST WITH (2) 1/2" DIA. GALV. BOLTS. ALTERNATE: PROVIDE COLUMN CAP CONNECTION WITH #4C SERIES BY SIMPSON STONE-TIE OR EQ. PROVIDE SOLID BLOCKING BELOW ALL COLUMNS, TO TRANSFER LOAD DIRECTLY TO FRAMING/FOUNDATION BELOW.

PROVIDE DOUBLE JOIST UNDER ALL PARTITIONS PARALLEL TO JOIST SPAN AND AROUND ALL FLOOR AND ROOF OPENINGS. SPACE & BLOCK IF PARTITIONS ABOVE IS A PLUMBING WALL. PROVIDE SOLID BLOCKING AT 12" @ BETWEEN JOISTS UNDER PARTITIONS ABOVE WHICH ARE PARALLEL TO THE JOISTS BUT NOT DIRECTLY OVER THE JOISTS. BLOCKING SHALL BE NOT LESS THAN 2" IN THICKNESS & SHALL MATCH THE DEPTH OF THE JOISTS. TRUSSES MAY USE TRUSS BLOCKS.

ALL MULTI-PLY BEAMS SHALL BE NAILED WITH 3 ROWS OF 10d NAILS AT 8" @ STAGGERED OR BOLTED WITH 1/2" DIA. BOLTS AT 16" @ STAGGERED (U.N.O.).

PROVIDE COLLAR TIES OF 1x6 BOARDS AT UPPER 1/3 DOWN FROM RIDGE BEAM'S SPACED 48" @ MAXIMUM. (FOR CONVENTIONAL FRAMING)

BALLOON FRAME ALL END WALLS WITH CATHEDRAL CEILING (U.N.O.):  
 2x4 @ 16" @ UP TO 9'-0", 2x6 @ 16" @ UP TO 14'-0" & 2x8 @ 16" @ UP TO 18'-0"

FASTEN GABLE-END WALL STUDS TO CEILING DIAPHRAGM BY FASTENING NAILER TO EACH STUD AND BY FASTENING CEILING TO NAILER WITH 8d NAILS AT 6" @

WHERE DECKS FASTEN TO HOUSE FRAMING, PROVIDE CONTINUOUS TREATED LEDGER THRU-BOLTED TO FLOOR STRUCTURE WITH (2) 1/2" DIA. BOLTS AT 16" @ PROVIDE HOT-DIPPED GALV. JST. HANGER TO LEDGER.

ALL EXTERIOR WALLS SHALL BE STUDS AT 16" @ AS SPECIFIED ON PLANS WITH 7/16" OSB EXTERIOR SHEATHING. BLOCKING OF HORIZONTAL PANEL EDGES IS NOT REQUIRED. NAIL ALL REQUIRED PANEL EDGES WITH 8d NAILS AT 6" @ AND INTERMEDIATE STUDS WITH 8d NAILS AT 12" @

ROOF AND FLOOR FRAMING LAYOUTS ARE PROVIDED TO ILLUSTRATE CONDITIONS OF CONSTRUCTION AND DO NOT NECESSARILY INDICATE SPECIFIC QUANTITIES OF MATERIALS OR COMPONENTS REQUIRED FOR CONSTRUCTION.

CONSTRUCTION BRACING SHALL BE PROVIDED BY THE CONTRACTOR TO MAINTAIN THE BUILDING PLUMB AND TRUE. THIS BRACING SHALL REMAIN UNTIL THE SPECIFIED SHEARWALLS ARE TOTALLY INSTALLED.

PRESCRIPTIVE BRACED WALL SEGMENTS SHALL HAVE STUDS AT 16" @ (MAX.) WITH 2" @ OSB EXTERIOR SHEATHING. BLOCKING OF HORIZONTAL PANEL EDGES IS NOT REQUIRED. NAIL ALL SHEATHING PANEL EDGES WITH 8d NAILS AT 6" @ AND INTERMEDIATE STUDS WITH 8d NAILS AT 12" @

SHEARWALLS SHALL HAVE STUDS @ 16" @ (MAX.) WITH 2" @ OSB EXTERIOR SHEATHING (U.N.O., SEE PLAN). BLOCKING OF HORIZONTAL PANEL EDGES IS REQUIRED. NAIL ALL SHEATHING PANEL EDGES WITH 8d NAILS AT 6" @ (U.N.O., SEE PLAN) AND INTERMEDIATE STUDS WITH 8d NAILS AT 12" @ (U.N.O., SEE PLAN)

SHEAR WALL HOLD-DOWNS: ALL SHEAR WALLS SHOWN ON PLANS TO HAVE HOLD-DOWNS AT THE BASE AT EACH WALL END SHALL BE AS FOLLOWS:  
 \* AT UPPER FLOORS USE (2) SIMPSON HDBA'S OR (1) SIMPSON FTAT AT EACH END OF SHEAR WALL SEGMENT AND EACH EXTERIOR CORNER OF BUILDING (U.N.O., SEE PLAN)  
 \* AT CONCRETE FOUNDATIONS USE (1) SIMPSON HDBA AT EACH END OF SHEAR WALL SEGMENT AND AT EACH EXTERIOR CORNER OF BUILDING (U.N.O., SEE PLAN)  
 \* AT PILE/GIRDER SUPPORTED FLOOR, USE (2) SIMPSON HDBA'S OR (1) SIMPSON FTAT AT EACH END OF SHEAR WALL SEGMENT AND AT EACH EXTERIOR CORNER OF BUILDING (U.N.O., SEE PLAN)

\* PROVIDE 3 STUDS MIN. AT EACH HOLD-DOWN (U.N.O., SEE PLAN)  
 \* PROVIDE TRIPLE JOISTS BELOW SHEAR WALLS THAT RUN PARALLEL TO FLOOR FRAMING (U.N.O., SEE PLAN)

ALL INTERIOR SHEAR WALLS SHOWN ON THE PLANS SHALL HAVE STRUCTURAL SHEATHING THAT EXTENDS TO THE UNDERSIDE OF THE FLOOR SHEATHING ABOVE WHERE JOISTS RUN PARALLEL TO THE SHEAR WALL, PROVIDE A DEL.-JOIST ABOVE THE SHEAR WALL. WHERE JOISTS RUN PERPENDICULAR, PROVIDE 2X BRIDGING ABOVE SHEAR WALL AND "TOOTH" PLYWOOD AROUND JOISTS. NAIL THROUGH FLOOR SHEATHING ABOVE INTO WALL WITH (2) 10d NAILS AT 4" @

ALTERNATE POWER NAILS (FOR FRAMING MEMBERS ONLY, - 0.139 x 2 3/8" FOR 8d NAILS & 0.1319 x 3" FOR 16d NAILS  
 PROVIDE DEFORMED SHANK NAILS AS REGD. BY U.L. RATINGS.

**STEEL**

FABRICATION AND ERECTION OF ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE LATEST SPECIFICATION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION.

PROVIDE WELDED CONNECTIONS TYPICALLY UNLESS OTHERWISE NOTED.

WELDS SHALL BE MADE ONLY BY WELDERS WHO HAVE BEEN PREQUALIFIED BY TESTS OF THE AMERICAN WELDING SOCIETY, PRESCRIBED IN THE STRUCTURAL WELDING CODE, AWS D11 (LATEST EDITION).

ANY CONNECTION NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED AND DETAILED BY THE STRUCTURAL STEEL FABRICATOR. SEE THE TYPICAL BEAM CONNECTION DETAILS ON THE DRAWINGS.

MILL BOTTOM OF ALL COLUMNS AND FINISH TOP OF ALL BASE PLATES IN ACCORDANCE WITH A.I.S.C. SPECIFICATIONS. BASE PLATES SHALL BE WELDED TO BOTTOM OF COLUMNS.

CONNECTIONS SHALL BE AISC STANDARD.

PROVIDE BASE PLATE FOR ALL STRUCTURAL STEEL BEAMS BEARING ON CONCRETE OR MASONRY. GROUT FOR SETTING BEARING SURFACES SHALL BE NON-SHRINK, NOT-STAINING, EQUAL TO "MASTERFLO 713" BY THE MASTER BUILDERS CORPORATION.

SPECIFIED GROUT THICKNESS INCLUDES 1/4" INCH THICK LEVELING PLATES WHICH SHALL BE USED UNDER ALL BEAMS AND COLUMNS RESTING ON CONCRETE.

**SCHEDULE OF CONSTRUCTION MATERIALS**

CONCRETE	LOCATION	COMP. STRENGTH	SLUMP
	BASEMENT WALLS & FDN NOT EXPOSED TO WEATHER	3000 PSI (2)	4" +/- 1"
	BASEMENT SLABS AND INTERIOR SLABS ON GRADE	3000 PSI	4" +/- 1"
	BASEMENT WALLS, FDNS, EXTERIOR WALLS & OTHER CONCRETE EXPOSED TO WEATHER	3000 PSI (3)	4" +/- 1"
	DRIVEWAYS, CURBS, WALKS, PATIOS, STEPS AND UNHEATED GARAGE FLOORS EXPOSED TO WEATHER	3500 PSI (3)	4" +/- 1"

NOTES:  
 1) THE COMPRESSIVE STRENGTH IS BASED 28-DAY CONDITIONS STRENGTH.  
 2) CONCRETE SUBJECTED TO FREEZE AND THAW CONDITIONS DURING CONSTRUCTION SHALL BE AIR-ENTRAINED (4% +/- .1%).  
 3) CONCRETE SHALL BE AIR-ENTRAINED (6% +/- .1%).

MASONRY	MATERIAL	SPECIFICATION
	HOLLOW CMU	NORMAL HEIGHT: ASTM C90, GRADE N, Fm= 1500 PSI
	FACE BRICK	ASTM C216, SEVERE WEATHER BRICK, TYPE BV, Fm=2000 PSI
	STONE VENEER	OWNER APPROVED
	CONCRETE BRICK	ASTM C98 TYPE I, GRADE 0
	SOLID CMU	NORMAL HEIGHT: ASTM C45, GRADE N
	MORTAR: SINGLE W/TH ABOVE GRADE	ASTM C270 PROJECTION SPECIFICATION MORTARS SHALL CONSIST OF TYPE I PORTLAND CEMENT, TYPE II HYDRATED LIME AND APPROVED AGGREGATE, WITH 1000 PSI MINIMUM AVERAGE COMPRESSIVE STRENGTH OF 2-INCH CUBES AT 28-DAYS.
	MORTAR: SINGLE W/TH BELOW GRADE	ASTM C270 PROJECTION SPECIFICATION MORTARS SHALL CONSIST OF TYPE I PORTLAND CEMENT, TYPE II HYDRATED LIME AND APPROVED AGGREGATE, WITH 2000 PSI MINIMUM AVERAGE COMPRESSIVE STRENGTH OF 2-INCH CUBES AT 28-DAYS.
	MORTAR: VENEER	ASTM C270 PROJECTION SPECIFICATION MORTARS SHALL CONSIST OF TYPE I PORTLAND CEMENT, TYPE II HYDRATED LIME AND APPROVED AGGREGATE, WITH 750 PSI MINIMUM AVERAGE COMPRESSIVE STRENGTH OF 2-INCH CUBES AT 28-DAYS.

REINFORCING STEEL	MATERIAL	SPECIFICATION
	REBAR	HIGH STRENGTH HEN BULLEET STEEL CONFORMING TO ASTM A-615, GRADE 60 (60,000 PSI) - DEFORMED
	WELDED WIRE FABRIC	ASTM A-95

	LOCATION	PROTECTION	CLEAR COVER (IN)
	FOOTINGS AND OTHER CONCRETE POURED AGAINST EARTH		3"
	FORMED CONCRETE EXPOSED TO EARTH		2"
	FORMED CONCRETE NOT EXPOSED TO WEATHER OR EARTH		1 1/2"
	SLABS ON GROUND, UNLESS OTHERWISE NOTED		MID-DEPTH OF SLAB
	REINFORCED MASONRY WALLS		MID-DEPTH OF WALL

STRUCTURAL STEEL	SHAPE	SPECIFICATION
	I-BEAMS	STRUCTURAL STEEL I BEAMS SHALL CONFORM TO ASTM A572 GRADE 50 (50 KSI)
	TUBE	STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A500, GRADE B, UNLESS OTHERWISE NOTED IN THE PROJECT SPECIFICATIONS
	PIPE	STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A36 (36KSI), UNLESS OTHERWISE NOTED IN THE PROJECT SPECIFICATIONS
	ALL OTHER SHAPES	ALL OTHER STRUCTURAL STEEL, INCLUDING PLATES AND MISCELLANEOUS SHAPES SHALL CONFORM TO ASTM A36 (36KSI)
	CONNECTION	
	BOLTS	BOLTS FOR CONNECTING STRUCTURAL STEEL SHAPES SHALL BE ASTM A325-N, 3/4-INCH DIAMETER, UNLESS OTHERWISE NOTED ON THE DRAWINGS OR IN THE PROJECT SPECIFICATION
	ANCHOR BOLTS	ANCHOR BOLTS SHALL CONFORM TO ASTM A307.
	WELDS	WELDING ELECTRODES SHALL BE E70 SERIES.

WOOD	MATERIAL	DIMENSION AND STRUCTURAL COMPOSITE LUMBER DESIGN VALUES (1)					
		F <sub>b</sub>	F <sub>t</sub>	F <sub>v</sub>	F <sub>c</sub> I	F <sub>c</sub> II	E x 10 <sup>6</sup>
UNTREATED FRAMING (2)	2X, 3X, OR 4X	875	450	195	425	1850	1.4
	6X6 AND LARGER (B)	800	300	125	425	425	1.0
	6X6 AND LARGER (P)	500	325	125	425	500	1.0
TREATED FRAMING (3)	2X4	500	825	175	565	1650	1.6
	2X6	1250	725	175	565	1600	1.6
	2X8	1000	650	175	565	1550	1.6
	2X10	1050	575	175	565	1500	1.6
	2X12	975	550	175	565	1450	1.6
	6X6 AND LARGER	850	550	165	375	525	1.2
	LVL COLUMNS AND BEAMS	2650	1650	285	750	3000	1.7
	LVL (2.0E) BEAMS	3100	2150	285	750	3000	2.0

MATERIAL	STRUCTURAL GLUED LAMINATED TIMBER (GLULAM) DESIGN VALUES (1)						
	F <sub>b</sub> (KSI)	F <sub>t</sub> (KSI)	F <sub>v</sub> (KSI)	F <sub>c</sub> I (KSI)	F <sub>c</sub> II (KSI)	E x 10 <sup>6</sup>	
UNTREATED FRAMING	BEAMS 24F-1.8E	2400	850	240	650	1600	1.8
	COLUMNS 24F	1700	1800	180	560	1950	1.6

MATERIAL	ENGINEERED WOOD PRODUCTS					
	F <sub>b</sub> (KSI)	F <sub>t</sub> (KSI)	F <sub>v</sub> (KSI)	F <sub>c</sub> I (KSI)	F <sub>c</sub> II (KSI)	E x 10 <sup>6</sup>
PREFABRICATED WOOD I-JOISTS	PREFABRICATED WOOD I-JOISTS SHALL BE MANUFACTURED BY BOISE CASCADE, LLC, OR APPROVED SUBSTITUTE. THE MANUFACTURER SHALL SUPPLY ALL REQUIRED HANGERS, WEB STIFFENERS, SOAKING BLOCKS, BEVELLED BEARING PLATES, AND OTHER SPECIAL HARDWARE. THE MANUFACTURER SHALL SUBMIT ERECTION DRAWINGS TO THE ENGINEER PRIOR TO FABRICATION. ALL PREFABRICATED WOOD I-JOISTS SHALL BE INSTALLED AND BRACED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS.					
PLYWOOD/OSB	DOC P51, DOC P52, CSA407 OR CSA408 ADVANTAGE, STRUCTURE WOOD NOT ALLOWED.					

NOTES:  
 1) DESIGN VALUES ARE FOR NORMAL LOAD DURATION AND DRY SERVICE CONDITIONS. SEE NDS OR MANUFACTURER'S SPECIFICATION FOR A COMPREHENSIVE DESCRIPTION OF DESIGN VALUE ADJUSTMENT FACTORS.  
 2) FRAMING DESIGN VALUES ARE BASED ON SFP No.2  
 3) FRAMING DESIGN VALUES ARE BASED ON SFP No.2

**GOVERNING BUILDING CODES AND STANDARDS**

THE FOLLOWING CODES AND STANDARDS, INCLUDING ALL SPECIFICATIONS REFERENCED WITHIN, SHALL APPLY TO THE DESIGN, CONSTRUCTION, QUALITY CONTROL AND SAFETY OF ALL WORK PERFORMED ON THE PROJECT. USE THE LATEST EDITIONS UNLESS NOTED OTHERWISE.  
 \* INTERNATIONAL RESIDENTIAL CODE (IRC), INTERNATIONAL CODE COUNCIL, INC., 2015  
 \* MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES (ANSI/ASCE 07-10), AMERICAN SOCIETY OF CIVIL ENGINEERS  
 \* BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318-11, AMERICAN CONCRETE INSTITUTE  
 \* ACI MANUAL OF CONCRETE PRACTICE - PARTS I THROUGH 5 - 2011  
 \* MANUAL OF STANDARD PRACTICE, CONCRETE REINFORCING STEEL INSTITUTE.  
 \* MANUAL OF STEEL CONSTRUCTION - 13TH EDITION, AMERICAN INSTITUTE OF STEEL CONSTRUCTION (INCLUDING SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS, AND AISC CODE OF STANDARD PRACTICE WITH EXCEPTION, IF ANY, AS INDICATED IN THE SPECIFICATIONS).  
 \* MANUAL OF STEEL CONSTRUCTION, VOLUME II CONNECTIONS, ASD 13TH EDITION/LRFD 1ST EDITION, AMERICAN INSTITUTE OF STEEL CONSTRUCTION.  
 \* DETAILING FOR STEEL CONSTRUCTION, AMERICAN INSTITUTE OF STEEL CONSTRUCTION.  
 \* STRUCTURAL WELDING CODE ANSI/AWS D 11-92, AMERICAN WELDING SOCIETY.  
 \* DESIGN MANUAL FOR FLOOR DECKS AND ROOF DECKS, STEEL DECK INSTITUTE.  
 \* SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS AMERICAN IRON AND STEEL INSTITUTE, AISI 900-2007.  
 \* BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530.1-05/ASCE 5-05/TMS 402-05) & SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530.1-05/ASCE 5-05/TMS 402-05)  
 \* NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION/ 2012, AMERICAN FOREST & PAPER ASSOCIATION.

**DESIGN LOADS**

	LIVE LOADS	DEAD LOADS	TOTAL
ROOF TRUSSES	30 PSF	10 PSF (TOP & BOTTOM)	50 PSF
RAFTERS	30 PSF	12 PSF	42 PSF
ATTIC FLOORS (TYP)	30 PSF	12 PSF	42 PSF
LTD STORAGE	20 PSF	12 PSF	32 PSF
NO STORAGE	10 PSF	5 PSF	15 PSF
SLEEPING ROOMS	30 PSF	12 PSF	42 PSF
OTHER FLOORS	40 PSF	12 PSF	52 PSF
GARAGE FLOORS	50 PSF	10 PSF	100 PSF
DECKS	40 PSF	10 PSF	50 PSF
BALCONY	60 PSF	10 PSF	70 PSF
STAIRS	40 PSF	20 PSF	60 PSF

ROOF LIVE LOAD	DESIGN MINIMUM	30 PSF
WIND LOAD	BASIC WIND SPEED (3 SEC GUST)	115 MPH
	WIND PRESSURE (ROOF AVG.)	10.0 PSF
	WIND PRESSURE (WALL AVG.)	27.9 PSF
	WIND LOAD IMPORTANCE	1.0
	WIND EXPOSURE CATEGORY	EXPOSURE B

SNOW LOAD	GROUND SNOW LOAD (P <sub>s</sub> )	30 PSF
	THERMAL FACTOR	1.1
	SNOW EXPOSURE FACTOR (C <sub>e</sub> )	1.0

CONCRETE

ALL CONCRETE SHALL BE MADE IN ACCORDANCE WITH DESIGN MIXES WHICH ARE TO BE APPROVED BY THE ARCHITECT OR ENGINEER PRIOR TO CASTING ANY CONCRETE. MIXES SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE ACI 318.1 AND ACI 309.1 GUIDE TO RESIDENTIAL CAST IN PLACE CONCRETE CONSTRUCTION. MIXES SHALL HAVE A MINIMUM CEMENT CONTENT OF 520 LB. PER CUBIC YD., MAXIMUM WATER/CEMENT RATIO OF 0.53 FOR INTERIOR CONCRETE PROTECTED FROM FREEZING AND 0.46 FOR ALL EXTERIOR EXPOSED CONCRETE.

CONCRETE MATERIALS SHALL CONFORM TO ASTM C150, TYPE I FOR PORTLAND CEMENT AND ASTM C33 FOR AGGREGATES. WATER-REDUCING ADMIXTURES SHALL CONFORM TO ASTM C494, TYPE A (FREE OF CALCIUM CHLORIDES), AIR-ENTRAINING ADMIXTURES SHALL CONFORM TO ASTM C260, AND HIGH-RANGE WATER REDUCERS (SUPER-PLASTICIZERS) SHALL CONFORM TO ASTM C494, TYPE F. FLY ASH SHALL COMPLY WITH ASTM C69 FOR CLASS F AND SHALL NOT BE PROPORTIONED IN MIXES WITH MORE THAN 20% CEMENT BY WEIGHT. LIQUID-MEMBRANE CURING COMPOUNDS SHALL BE HIGH-SOLIDS, WATER AND ACRYLIC-BASED, COMPLYING WITH ASTM C699 AS TESTED UNDER ASTM C156. SLUMP OF THE CONCRETE SHALL BE A MINIMUM OF 4-INCHES AND A MAXIMUM OF 6-INCHES. SEE THE PROJECT SPECIFICATIONS. THE COMPRESSIVE STRENGTH IS BASED 28-DAY COMPRESSIVE STRENGTH.

SLAB ISOLATION JOINTS: PROVIDE PRE-MOLDED JOINT FILLER AROUND ALL PIPING, PIERS & FOUNDATION WALLS.

ALL CONCRETE TO BE PLACED IN THE CELLS OF CONCRETE MASONRY UNITS (CMU BLOCK FILL), OR IN THE VOIDS OF BRICK MASONRY CONSTRUCTION, SHALL CONTAIN PEA GRAVEL (3/8" Ø STONE) IN LIEU OF COARSE AGGREGATE. THE CONCRETE MIX SHALL CONTAIN A HIGH-RANGE WATER REDUCER (SUPER-PLASTICIZER). SLUMP OF THE CONCRETE SHALL BE A MINIMUM OF 6" AND A MAXIMUM OF 9". SEE THE PROJECT SPECIFICATIONS.

ALL EXTERIOR CONCRETE AND CONCRETE EXPOSED TO WEATHER SHALL BE AIR-ENTRAINED, 6% +/- 1%. USE OF ADDITIVES SHALL NOT BE PERMITTED UNLESS SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER. USE OF ADDITIVES CONTAINING CALCIUM CHLORIDE SHALL NOT BE PERMITTED. DO NOT USE HIGH-RANGE WATER REDUCING ADMIXTURES IN AIR-ENTRAINED CONCRETE. CONFORM TO ASTM C260.

ADDITION OF WATER TO THE CONCRETE AT THE JOB SITE FOR THE PURPOSE OF INCREASING THE SLUMP OR FOR RETEMPERING THE CONCRETE WHICH HAS BEGUN TO SET IS STRICTLY PROHIBITED. SEE THE PROJECT SPECIFICATIONS FOR REQUIREMENTS OF WATER ADDITION TO CONCRETE AT THE JOBSITE.

SLABS ON GRADE SHALL BE 4" THICK CONCRETE AND REINFORCED W/ #6 @ 18" X 18" W/ #4 W/ HELDED WIRE FABRIC SHALL BE SUPPORTED ON HIGH CHAIRS SO THAT THE FABRIC IS POSITIONED AT MID-DEPTH OF THE SLAB THICKNESS. LAP ONE FULL MESH PLUS 2" AT SPLICES IN EACH DIRECTION. PLACE CONCRETE OVER 6 MIL. POLYETHYLENE VAPOR BARRIER AND 4" MINIMUM COURSE AGGREGATE OR AS RECOMMENDED BY SOILS ENGINEER. THE AGGREGATE LAYER SHALL BE PLACED OVER FIRM NATURAL SUBGRADE OR ON COMPACTED AND CONTROLLED FILL. FILL UNDER SLABS SHALL BE COMPACTED IN 6" INCH LAYERS TO 95% MAX. DENSITY. USE AIR-ENTRAINED AT ALL EXTERIOR SLABS.

CONCRETE FOR SLABS-ON-GRADE SHALL BE PLACED IN A SEQUENCE AND MANNER THAT IS CONSISTENT WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE. LOCATE CONSTRUCTION AND CONTROL JOINTS IN SUCH A WAY TO MINIMIZE THE EFFECTS OF SHRINKAGE OF THE CONCRETE SLAB SECTIONS. SUBMIT TO THE ARCHITECT/ENGINEER THE SEQUENCE AND METHOD OF CASTING CONCRETE SLABS-ON-GRADE PRIOR TO PLACING THESE ELEMENTS. POUR SLABS IN ALTERNATE PANELS WITH A MAXIMUM OF 600 SF AND PROVIDE CONTROL AND CONSTRUCTION JOINTS AT 15'-0" MAXIMUM OR AS REQUIRED TO PREVENT UNCONTROLLED CRACKING.

SLAB CONTROL JOINTS: SAW CUT OR FORM TO 1/3 SLAB DEPTH. SPACE NO MORE THAN 15 FEET APART. DISCONTINUE WELDED WIRE FABRIC AT SLAB CONTROL JOINTS. PROVIDE JOINTS ON GROUND SUPPORTED SLABS IN RECTANGULAR CONFIGURATION, WITH THE LONGER SIDE NO MORE THAN ONE-AND-ONE-HALF TIMES THE LENGTH OF THE SHORTER SIDE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR FURNISHING AND INSTALLING ANCHOR BOLTS, CLIPS, INSERTS, CONNECTION PLATES, SLEEVES, SLOTS AND OTHER REQUIRED ITEMS IN ACCORDANCE WITH THE CONTRACT DRAWINGS, AND IN COOPERATION WITH OTHER TRADES PRIOR TO PLACING CONCRETE.

ALL REINFORCING SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH ACI'S MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES, (ACI-318). DETAILS OF REINFORCEMENT SHALL CONFORM TO ACI 318, ACI 315, AND CRSI STANDARDS.

ALL REINFORCING STEEL (INCLUDING WELDED WIRE FABRIC) SHALL BE SECURELY TIED AND ANCHORED IN PLACE TO PREVENT DISLOCATION DURING THE PLACING OPERATION.

REINFORCING STEEL SHALL BE CLEAN OF MUD, DEBRIS, LOOSE RUST, CEMENT, GROUT, OR ANY OTHER MATERIAL WHICH MAY INHIBIT THE BOND BETWEEN THE STEEL AND CONCRETE.

PROVIDE 8" X 8" CORNER BARS TO MATCH ALL HORIZONTAL REINFORCING IN WALLS AND FOOTINGS. ALL LAPS SHALL BE A MINIMUM OF 36 BAR DIAMETERS. PROVIDE DONNELS BETWEEN ALL FOOTINGS, WALLS AND PIERS TO MATCH SIZE AND SPACING OF VERTICAL REINFORCING.

DRY PACK SHALL CONSIST OF SIKA GROUT 212 OR APPROVED SUBSTITUTE. INSTALL PER MANUFACTURER'S RECOMMENDATIONS.

WOOD

FITCH BEAMS SHALL BE SIZED AS INDICATED ON THE DRAWINGS, USING #2 SPF MINIMUM AND A-36 STEEL PLATE. USE TWO ROWS OF 1/2" DIAMETER THROUGH BOLTS 2" FROM TOP AND BOTTOM, SPACED 16" OC AT TOP AND 32" OC AT THE BOTTOM. BEGIN BOLTING ROWS 6" FROM ENDS. STEEL FITCH PLATES MUST BE EITHER FULL LENGTH OR FULL MOMENT BUTT SPLICE.

WOOD EXPOSED TO THE ELEMENTS, WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND WOOD DESIGNATED "TREATED" SHALL BE #2 GRADE SOUTHERN PINE OR BETTER & PRESSURE IMPREGNATED WITH ALKALINE COPPER QUATERNARY (ACQ) IN ACCORDANCE WITH AMERICAN WOOD PRESERVERS ASSOCIATION (AWPA) STANDARD C2, WITH A MIN. RETENTION OF 0.40 LBS. PER CUBIC FOOT OF WOOD. MIN. DEPTH OF PENETRATION SHALL BE 2.5" OR 85% OF THE SAKWOOD.

ALL STUDS SHALL BE INSTALLED IN ACCORDANCE WITH IFOPA. MEMBERS ARE NOT TO BE DRILLED IN EXCESS OF NDS OR LOCAL CODE REQUIREMENTS, WHICHEVER IS MORE STRINGENT. ALL POSTS AND MULTIPLE STUDS SHALL BE RUN CONTINUOUSLY TO SOLID BEARING ON FOUNDATION WALL OR BEAMS, PROVIDE SOLID BLOCKING AT FLOORS. COLUMNS SHALL BE ADEQUATELY ANCHORED TO PREVENT INTERNAL DISPLACEMENT.

FRAME CHIMNEYS: FRAME CHIMNEYS SHALL BE CONSTRUCTED OF MINIMUM #2 SPF STUDS, MAXIMUM 16" OC USE 2 X 4 IF CHIMNEY EXTENDS LESS THAN 8' ABOVE ROOF, OTHERWISE USE 2 X 6. SHEATH WITH 1/2" APA OR APPROVED SUBSTITUTE RATED SHEATHING CONTINUOUS ACROSS PLATES AND JOISTS, GLUE, AND NAIL WITH 8D NAILS @ 6" OC SECURE TO ROOF. STUDS MUST BE CONTINUOUS ACROSS ROOF INTERSECTION.

NO STRUCTURAL MEMBER SHALL BE OMITTED, NOTCHED, CUT, BLOCKED OUT OR RELOCATED WITHOUT PRIOR APPROVAL BY THE DESIGNER OR STRUCTURAL ENGINEER. DO NOT ALTER SIZES OF MEMBERS NOTED WITHOUT APPROVAL OF BOTH.

CUTTING OF WOOD BEAMS, JOISTS AND RAFTERS SHALL BE LIMITED TO CUTS AND BORED HOLES NOT DEEPER THAN ONE-SIXTH THE MEMBER DEPTH AND SHALL NOT BE LOCATED WITHIN THE MIDDLE THIRD OF THE SPAN. NOTCHES LOCATED CLOSER TO SUPPORTS THAN THREE TIMES THE MEMBER DEPTH SHALL NOT EXCEED ONE-FIFTH THE DEPTH. HOLES BORED OR CUT INTO JOISTS SHALL BE MIN. 2" CLEAR FROM THE TOP OR BOTTOM OF THE JOIST AND THE HOLE DIAMETER SHALL NOT EXCEED ONE-THIRD OF THE JOIST DEPTH.

THERE SHALL NOT BE LESS THAN ONE LINE OF BRIDGING IN EVERY EIGHT FEET OF SPAN IN FLOOR, ATTIC AND ROOF FRAMING. THE BRIDGING SHALL CONSIST OF NOT LESS THAN ONE BY THREE INCH LUMBER DOUBLE NAILLED AT EACH END OR OF EQUIVALENT METAL BRACING OF EQUAL RIGIDITY. MIDSPAN BRIDGING IS NOT REQUIRED FOR FLOOR, ATTIC OR ROOF FRAMING WHERE JOIST DEPTH DOES NOT EXCEED TWELVE INCHES NOMINAL. BLOCK ALL STUD WALLS AT MAXIMUM INTERVALS OF EIGHT FEET WITH A MINIMUM OF TWO-BY-FIVE SOLID MATERIAL WITH TIGHT JOINTS. PROVIDE TWO-BY-FIVE FIRE STOPS AT MID-POINT OF STUD WALLS.

UNLESS NOTED OTHERWISE, BRACE EXTERIOR CORNERS OF BUILDING WITH 1 X 4 DIAGONALS, LET INTO STUDS, OR 4 X 8 PLYWOOD SHEET OF THICKNESS TO MATCH THAT OF SHEATHING, OR WITH METAL STRAPS. LAP PLATES AT ALL CORNERS.

MISCELLANEOUS

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL SAFETY REGULATIONS, PROGRAMS AND PRECAUTIONS RELATED TO ALL WORK ON THIS PROJECT AND FOR THE PROTECTION OF PERSONS AND PROPERTY EITHER ON OR ADJACENT TO THE PROJECT AND SHALL PROTECT SAME AGAINST INJURY, DAMAGE OR LOSS.

THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED ON THE STRUCTURE. SUCH LOADS SHALL NOT EXCEED THE CAPACITY OF THE STRUCTURE AT ANY TIME. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION, AND ANY TEMPORARY BRACING OR SUPPORT REQUIRED TO ACCOMMODATE THE CONTRACTOR'S MEANS AND METHODS ARE THE RESPONSIBILITY OF THE CONTRACTOR.

THE CONTRACTOR IS TO VERIFY ALL OPENING SIZES AND LOCATIONS WITH THE REQUIREMENTS OF OTHER TRADES PRIOR TO FABRICATION AND ERECTION.

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS, AND THE CONTRACTOR SHALL BE RESPONSIBLE FOR SEEING THAT THE WORK OF ALL TRADES IS COORDINATED WITH STRUCTURAL WORK.

EARTH RETAINING WALLS, OTHER THAN CANTILEVERED TYPE WALLS, SHALL BE ADEQUATELY BRACED UNTIL CONCRETE FOR SUPPORTING SLABS HAS BEEN PLACED AND ALL CONCRETE HAS CURED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR DESIGNING, FURNISHING, ERECTING AND REMOVING ANY TEMPORARY SHORING AND BRACING DURING CONSTRUCTION. THE ARCHITECT AND ENGINEER SHALL BE NOTIFIED AT THE PROPER TIME WHEN ALL ITEMS ARE READY FOR OBSERVATION. SUFFICIENT NOTICE SHALL BE GIVEN BY THE CONTRACTOR TO ALLOW FOR SCHEDULING OF OBSERVATIONS.

SAFETY REGULATIONS SHALL BE STRICTLY FOLLOWED BY THE CONTRACTOR OR SUBCONTRACTOR DURING ALL TIMES OF WORK ON THIS PROJECT. THE ARCHITECT OR ENGINEER SHALL NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR ACTS OF OMISSIONS OF THE CONTRACTOR, SUBCONTRACTORS, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

ALL SPECIALTY BOLTS, INCLUDING EXPANSION TYPE AND EPOXY TYPE ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTRUCTIONS.

THE CONTRACTOR SHALL PROTECT FROM DAMAGES EXISTING BUILDING(S), OWNER EQUIPMENT, ROADS, WALKS AND UTILITIES. THE CONTRACTOR SHALL MAINTAIN THESE DURING THE COURSE OF THE WORK, AND SHALL REPAIR ALL DAMAGES AT NO ADDITIONAL EXPENSE TO THE OWNER. IN AREAS WHERE THE DRAWINGS DO NOT ADDRESS METHODOLOGY, THE CONTRACTOR SHALL BE BOUND TO PERFORM IN STRICT COMPLIANCE WITH MANUFACTURER'S SPECIFICATIONS AND/OR RECOMMENDATIONS.

ON-SITE VERIFICATION OF ALL DIMENSIONS AND CONDITIONS SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND HIS SUBCONTRACTORS. NOTED DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALE.

THE GENERAL NOTES AND TYPICAL DETAILS APPLY THROUGHOUT THE JOB UNLESS OTHERWISE NOTED OR SHOWN.

THE CONTRACTOR SHALL COMPARE AND COORDINATE ALL DRAWINGS. IF A DISCREPANCY EXISTS, HE SHALL PROMPTLY REPORT IT FOR PROPER ADJUSTMENT BEFORE PROCEEDING WITH THE WORK.

IN THE EVENT THAT CERTAIN FEATURES OF THE CONSTRUCTION ARE NOT FULLY SHOWN ON THE DRAWINGS, THEIR CONSTRUCTION SHALL BE OF THE SAME CHARACTER AS SIMILAR CONDITIONS THAT ARE SHOWN OR NOTED.

THESE PLANS ARE SUBJECT TO MODIFICATIONS AS NECESSARY TO MEET CODE REQUIREMENTS OR TO FACILITATE MECHANICAL, PLUMBING INSTALLATIONS OR TO INCORPORATE DESIGN IMPROVEMENTS.

DO NOT BUILD OVER GAS LINES OR ENCLOSE THE METER. CONSULT THE LOCAL GAS COMPANY PRIOR TO CONSTRUCTION.

CHIMNEY SHALL EXTEND AT LEAST 2 FEET HIGHER THAN ANY PORTION OF THE BUILDING WITHIN 10 FEET, BUT SHALL NOT BE LESS THAN 3 FEET ABOVE THE POINT WHERE IT PASSES THROUGH THE ROOF.

DECKS ARE NOT APPROVED FOR FUTURE HOT TUB INSTALLATION.

NO OPENING NOR ANY CHANGES IN SIZE, DIMENSION OR LOCATION SHALL BE MADE IN ANY STRUCTURAL ELEMENTS WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.

CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO ORDERING MATERIALS OR PROCEEDING WITH NEW WORK IN AREAS AFFECTED BY EXISTING CONDITIONS. STRUCTURAL ENGINEER SHALL BE INFORMED IN WRITING OF CONFLICTS BETWEEN EXISTING AND PROPOSED NEW CONSTRUCTION.

CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL DIMENSIONS SHOWN ON THE CONTRACT DOCUMENTS. INCONSISTENCIES ON THE STRUCTURAL DRAWINGS OR BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER CONTRACT, SHOP, FABRICATION, OR OTHER DRAWINGS OR INFORMATION SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH AFFECTED WORK.

THE STRUCTURAL INTEGRITY OF THE BUILDING IS DEPENDANT UPON COMPLETION ACCORDING TO PLANS AND SPECIFICATIONS. THE STRUCTURAL ENGINEER ASSUMES NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION. THE METHOD OF CONSTRUCTION AND SEQUENCE OF OPERATIONS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL SUPPLY ANY NECESSARY SHORING, BRACING, GUYS, ETC., TO PROPERLY BRACE THE STRUCTURE AGAINST WIND, DEAD AND LIVE LOADS UNTIL THE BUILDING IS COMPLETED ACCORDING TO THE PLANS AND SPECIFICATIONS.

CONTRACTOR SHALL NOT PLACE BACK FILL AGAINST BASEMENT WALLS UNTIL THE FLOOR SYSTEM IS COMPLETELY INSTALLED OR CONTRACTOR HAS PROVIDED ADEQUATE SHORING AND BRACING. ANY QUESTIONS REGARDING TEMPORARY SHORING REQUIREMENTS SHOULD BE FORWARDED TO THE STRUCTURAL ENGINEER FOR REVIEW.

FOUNDATION STEM WALL REINFORCEMENT SCHEDULE

Table with columns for Stem Wall Height, Length, and Reinforcement (GH, GP, SH, SP, SC, G1, G2, G3, G4, G5, G6, G7, G8, G9, G10). Includes notes for 8' and 9' wall heights and concrete strength.

FOUNDATION WALL STRIP FOOTING SCHEDULE

Tables for 8" and 10" concrete wall strip footing sizes and vertical reinforcement. Includes diagrams for footing cross-section and reinforcement details.

FOUNDATION WALL REINFORCEMENT SCHEDULE

Tables for 8" and 10" concrete wall vertical reinforcement. Includes diagrams for wall cross-section and reinforcement details.

ISOLATED FOOTING SCHEDULE

Tables for square and round footing specifications and reinforcement equivalents. Includes diagrams for footing cross-section and reinforcement details.

GARAGE GRADE BEAM SCHEDULE

Table for garage grade beam specifications and reinforcement. Includes diagrams for beam cross-section and reinforcement details.

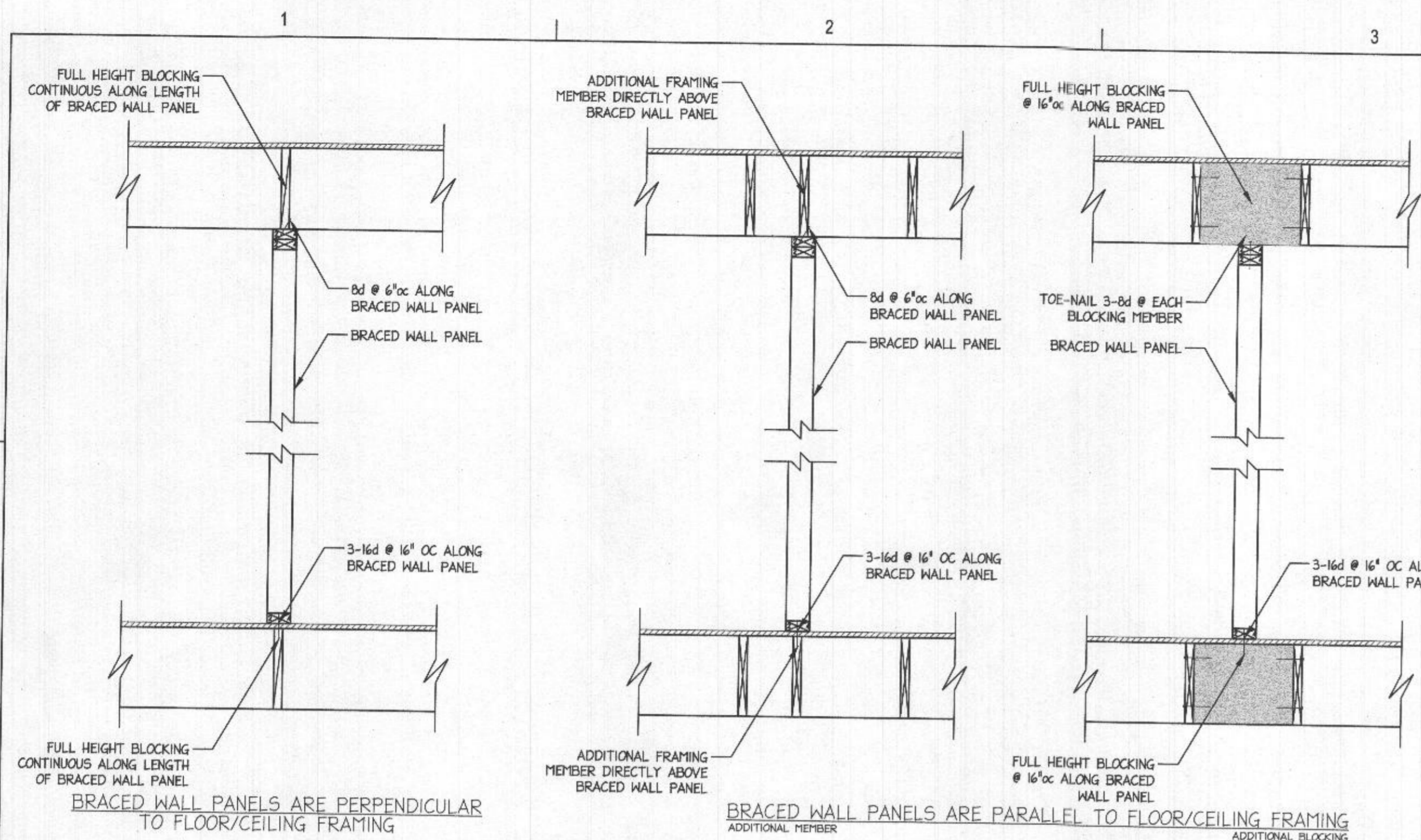
STRUCTURAL ENGINEERING UNLIMITED, LLC. 341 N. PATRICK STREET, FREDERICK, MD 21701. 240-815-6160. 301-148-2164.

THE OVERLOOK CABANA 15125 DEVLIN DR. GLENELG, MD 21737. LEVINBROHN & ASSOCIATES, INC. ARCHITECT. J PAL BUILDERS CONTRACTOR.

PROFESSIONAL CERTIFICATION: I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND. LICENSE NO. 24518. EXPIRATION DATE: 04-21-2021.

SCALE: AS NOTED. DRAWN BY: [initials]. CHECKED BY: [initials]. ISSUE: [blank]. DATE: 03-10-2021. REVISION: [blank].

GENERAL STRUCTURAL NOTES & SCHEDULES. S002.



### PARTIAL SHEATHING FASTENING SCHEDULE

SHEATHING	FASTENERS	S, SPACING OF FASTENERS	E, SPACING OF FASTENERS
2-PLY, 2x, TOP PLATE, EDGE NAILED			
1/2" - 3/4" PLYWOOD	6d COMMON, FLOOR, WALL & 8d COMMON, ROOF	6	12
5/8" - 1" PLYWOOD	8d COMMON	6	12
1 1/4" - 1 1/2" PLYWOOD	10d COMMON OR 8d DEFORMED	6	12
1/2" GYPSUM	1/2" GALV. ROOFING; 6d COMMON; 1/2" GALV. STAPLE; 1/2" SCREW, TYPE S OR W	4	8
5/8" GYPSUM	5/8" GALV. ROOFING; 6d COMMON; 5/8" GALV. STAPLE; 5/8" SCREW, TYPE S OR W	4	8

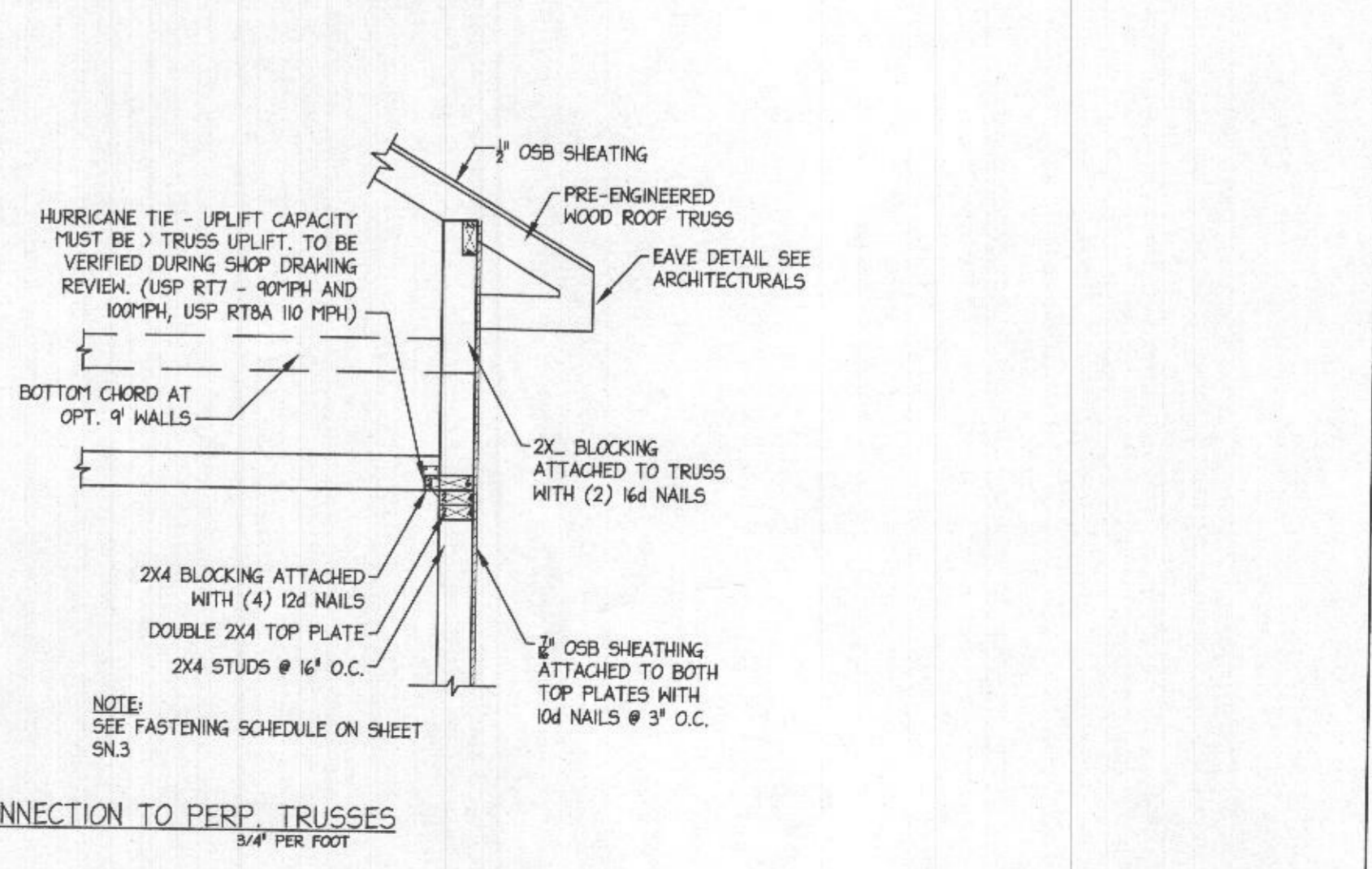
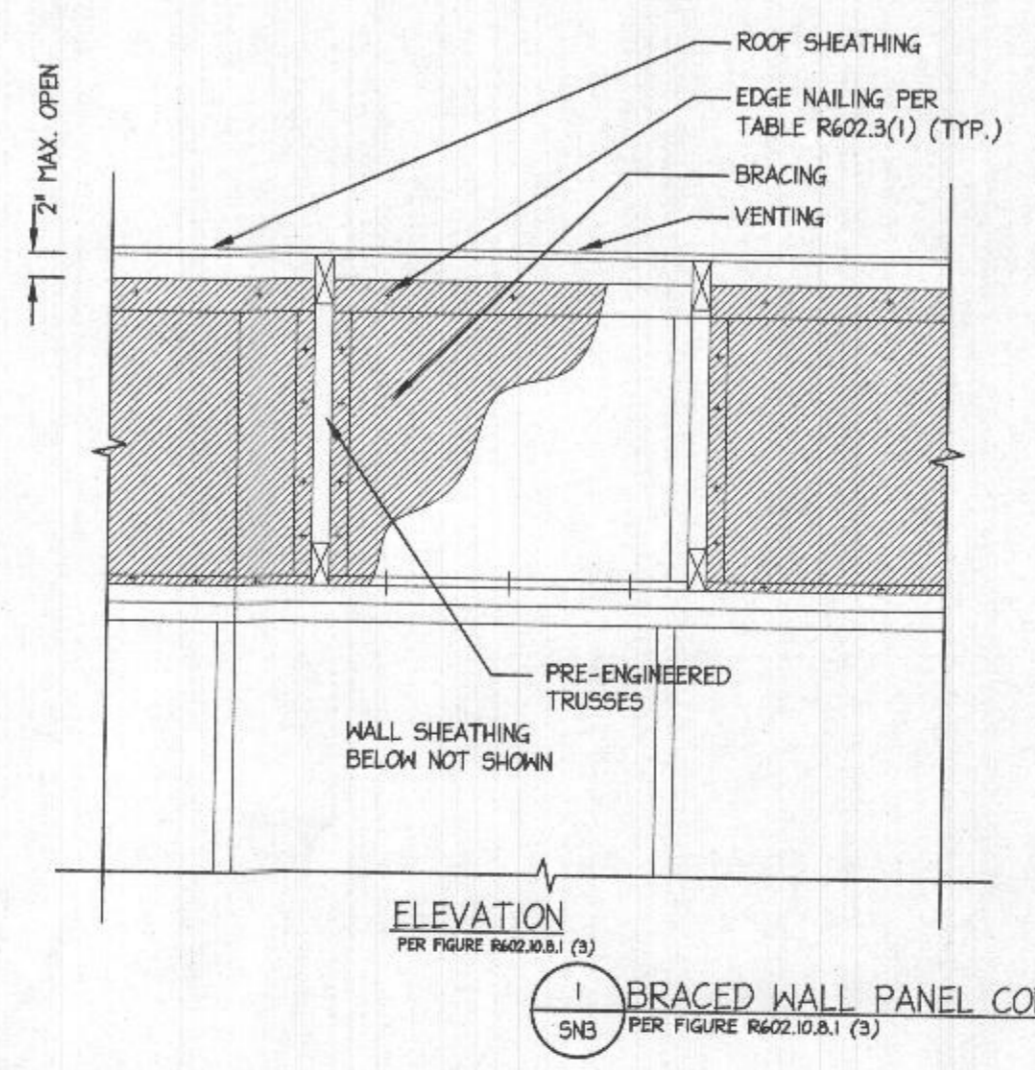
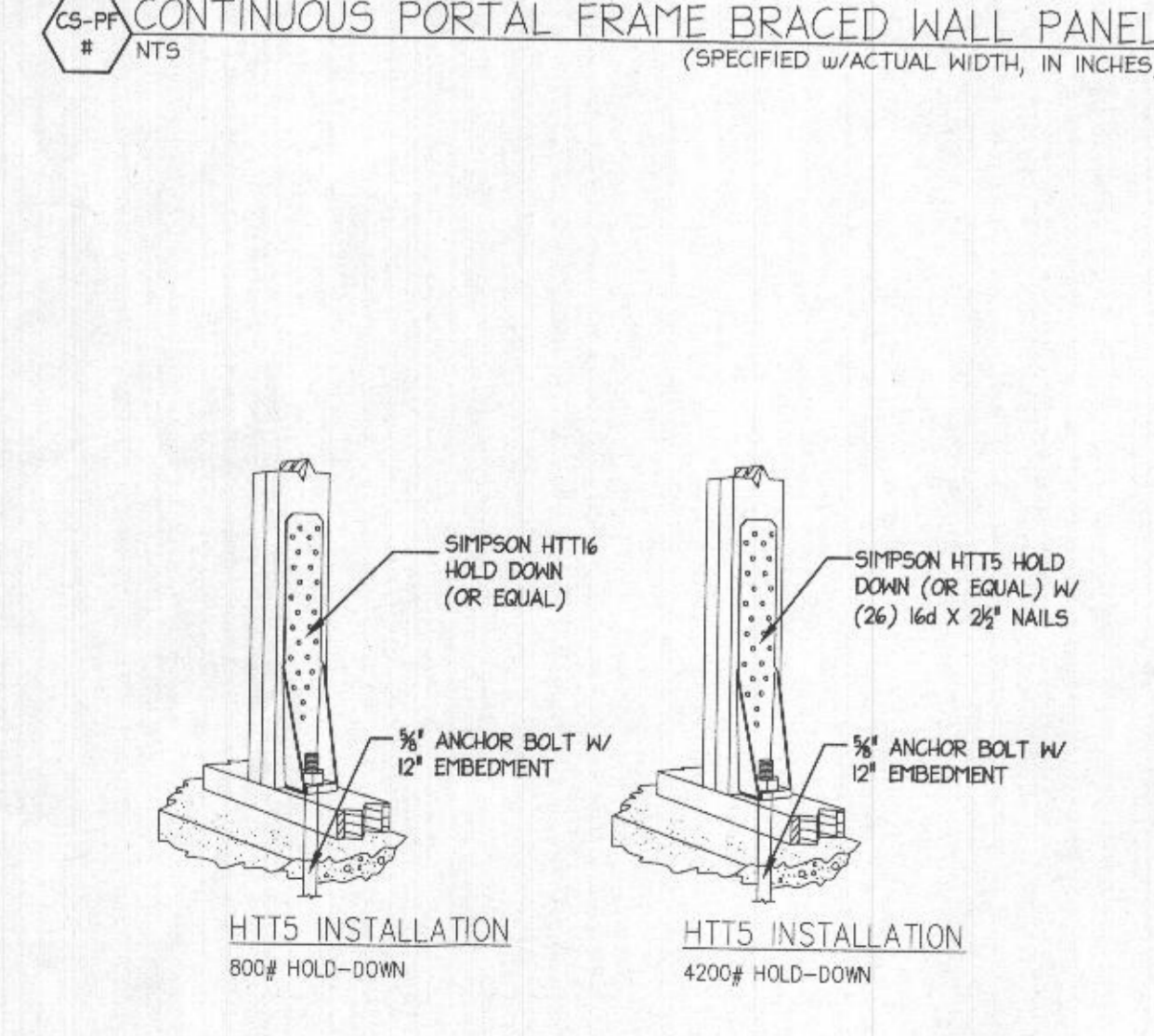
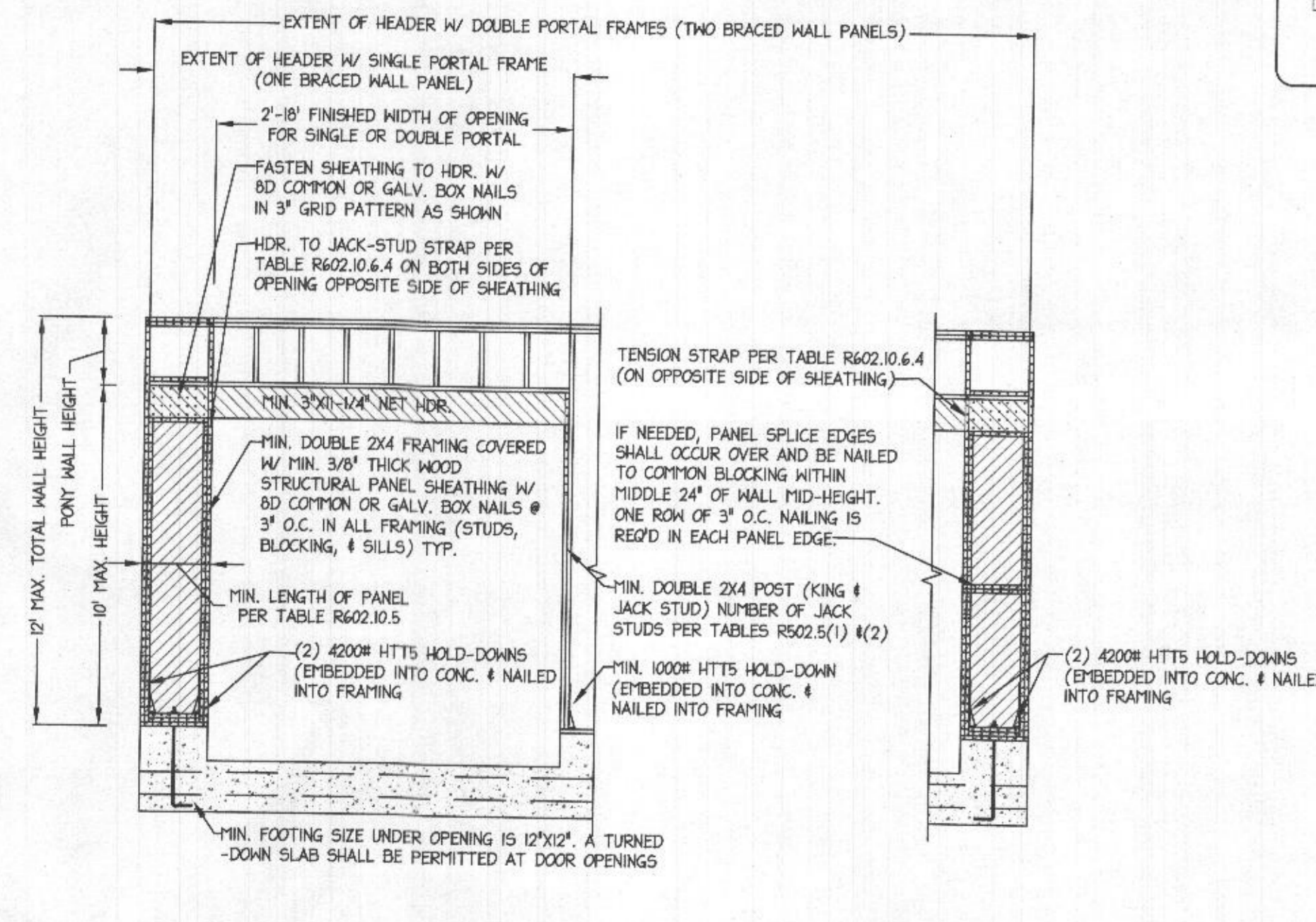
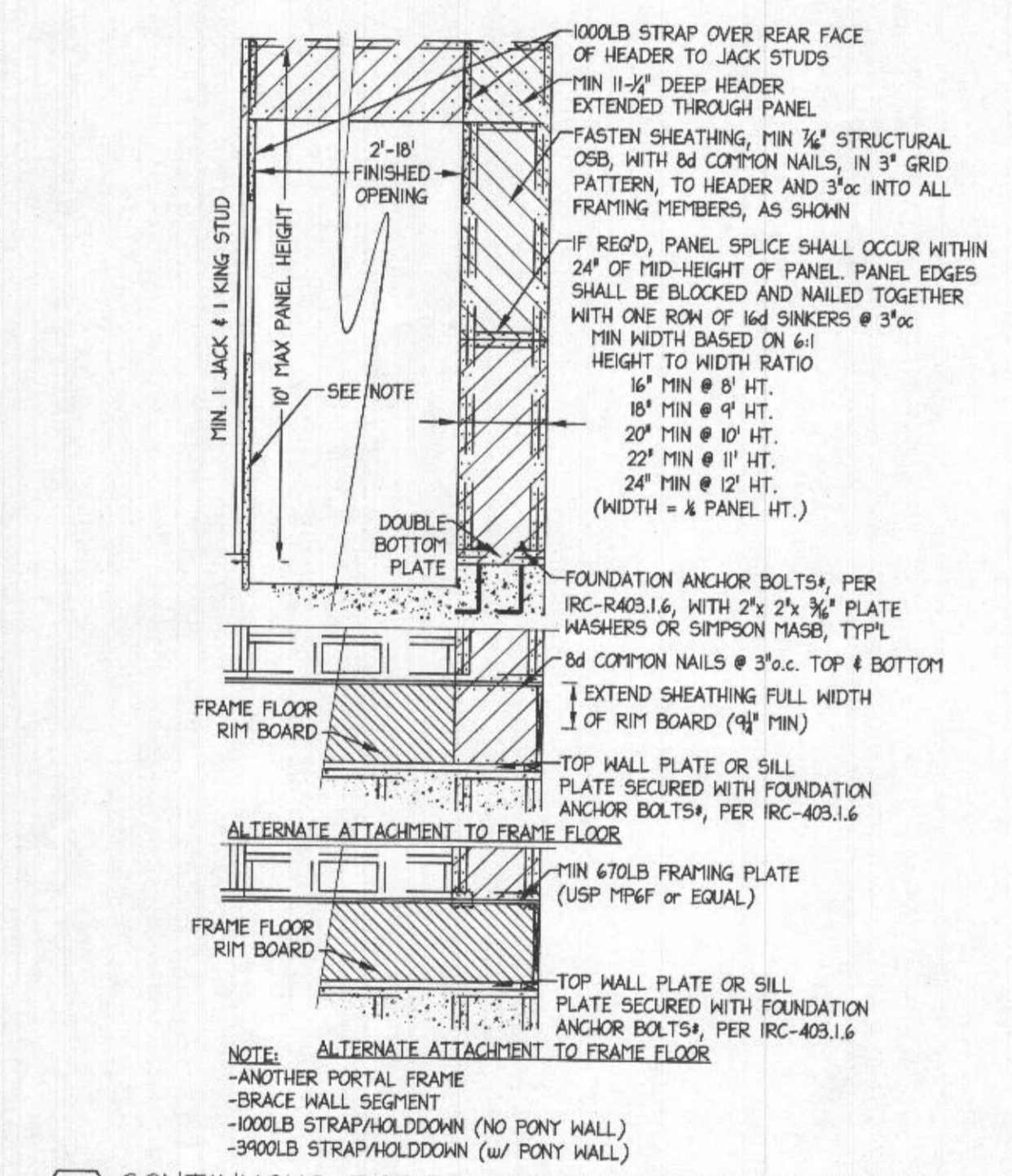
2x, BOTTOM PLATE, EDGE NAILED

2x, STUD, WALL PER FRAMING PLANS

DOUBLE NAIL EDGE SPACING

### PARTIAL FASTENING SCHEDULE

MARK	CONNECTION	FASTENING	DETAIL
1	TOP PLATE TO STUD, END NAIL	2x4 2-16d 2x6 3-16d 2x8 4-16d 2x10 5-16d 2x12 6-16d	(5)
2	DOUBLE TOP PLATE, FACE NAIL	10d @ 24"oc	(1, 2, 3)
3	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	16d TOENAILS @ 6"oc (MIN 2 PER BLOCK)	(4, 5, 6, 7)
4	CEILING JOISTS TO PLATE, TOE NAIL	(2) 16d	(1, 2)
5	CEILING JOIST/COLLAR TIE TO RAFTER, FACE NAIL	(6) 16d (MIN)	(1, 2)
6	RAFTER / TRUSS TO PLATE, TOE NAIL	(3) 16d	(1, 2)
7	BLOCKING TO JOIST OR RAFTER, EACH END	(2) 16d, TOE NAIL OR (2) 16d, END NAIL	(1, 2)
8	STUD TO SOLE PLATE, END NAIL	2x4 (2) - 16d 2x6 (3) - 16d 2x8 (4) - 16d 2x10 (5) - 16d 2x12 (6) - 16d	(1, 2, 3, 4, 5, 6)
9	SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL SEGMENTS, FACE NAIL	3-16d @ 16"oc 4-16d @ 19.2"oc OR 5-16d @ 24"oc	(1, 2, 3, 4, 5, 6)
10	SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	2-16d @ EACH JOIST OR BLOCKING	(1, 2, 3, 4, 5, 6)
11	SOLE PLATE TO RIM BOARD, FACE NAIL	16d @ 16"oc	(1, 2)
12	RIM BOARD TO TOP/ SILL PLATE, TOE NAIL	10d @ 6"oc	(1, 2)
13	JOIST TO RIM BOARD, END NAIL	(3) 16d	(1, 2, 3, 4, 5, 6)
14	JOIST TO TOP / SILL PLATE OR GIRDER, TOE NAIL	(2) 16d	(1, 2, 3, 4, 5, 6)
15	SILL PLATE TO FOUNDATION WALL	3/4" ANCHOR BOLTS (7" MIN EMBEDMENT INTO WALL) @ 48"oc (MAX) (MIN 2 PER PLATE, WITH 1 WITHIN 12" OF END OF PLATE)	(1, 2, 3, 4, 5, 6)
16	TOP PLATE LAPS SPLICE, FACE NAIL (4"-0" MINIMUM)	(8) 16d	(1, 2, 3, 4, 5, 6)
17	DOUBLE STUDS, FACE NAIL (STAGGER)	10d @ 12"oc EACH FACE	(1, 2, 3, 4, 5, 6)
18	JACK STUD TO KING STUD, FACE NAIL (STAGGER)	10d @ 12"oc EACH FACE	(1, 2, 3, 4, 5, 6)
19	KING STUD TO HEADER, FACE NAIL - EACH PLY	(3) 16d	(1, 2, 3, 4, 5, 6)
20	CONTINUED HEADER, TWO PIECES	16d @ 16"oc ALL EDGES & 4-16d NAILS AT ENDS	(1, 2, 3, 4, 5, 6)
21	BUILT UP HEADER, TWO PLYS WITH 1/2" SPACER	16d @ 16"oc ALL EDGES & 4-16d NAILS AT ENDS	(1, 2, 3, 4, 5, 6)
22	TOP PLATE LAP AT WALL INTERSECTION, FACE NAIL	(2) 10d	(1, 2, 3, 4, 5, 6)
23	CEILING JOIST TO JOIST, LAP OVER PARTITION	(5) 10d FACE NAILS	(1, 2, 3, 4, 5, 6)
24	RAFTER TO RIDGE, VALLEY OR HIP RAFTER	(3) 16d FACE NAILS, (4) 16d TOE NAILS	(1, 2, 3, 4, 5, 6)
25	BUILT-UP CORNER STUDS (THREE STUDS MINIMUM)	16d @ 16"oc	(1, 2, 3, 4, 5, 6)
26	BUILT-UP BEAM AND GIRDERS, 2-INCH LUMBER LAYERS, NAILING PER LAYER	16d @ 16"oc ALL EDGES & 4-16d NAILS AT ENDS AND SPLICES	(1, 2, 3, 4, 5, 6)
27	INTERMEDIATE SUPPORT POST TO HEADER, TOE NAIL	(2) 16d EACH PLY OF POST	(1, 2, 3, 4, 5, 6)



**STRUCTURAL ENGINEERING UNLIMITED, LLC**

341 N. PATRICK STREET  
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**THE OVERLOOK CABANA**  
15125 DEVLIN DR.  
GLENELG, MD 21737

LEVINEBROWN & ASSOCIATES, INC.  
ARCHITECT

J PAUL BUILDERS  
CONTRACTOR

PROFESSIONAL CERTIFICATION. I HEREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARED OR APPROVED BY ME, AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MARYLAND. LICENSE NO. 24818

EXPIRATION DATE: 09-21-2021



SCALE: AS NOTED

DRAWN BY: SL CHECKED BY: JH

ISSUE: DATE:

ISSUED FOR PERMITS: 03-10-2021

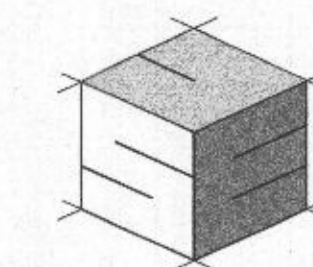
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**GENERAL STRUCTURAL NOTES & SCHEDULES**

**S003**

8/10/2021 6:29:04 AM



**STRUCTURAL ENGINEERING  
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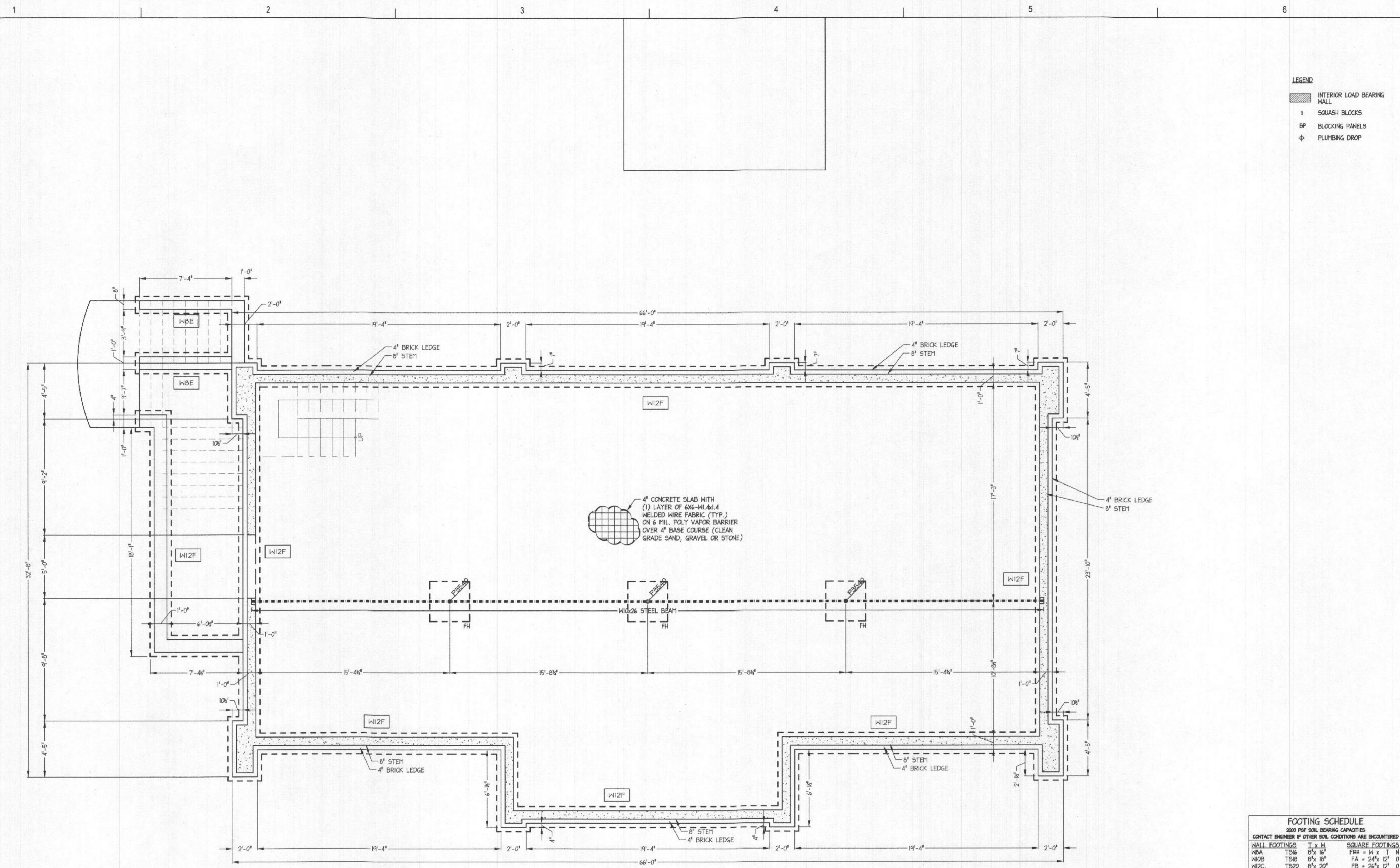
SCALE: AS NOTED  
DRAWN BY: SL CHECKED BY: JW

ISSUE: DATE:  
ISSUED FOR PERMITS 03-10-2021

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**FOUNDATION PLAN**

**S004**



**1 FOUNDATION PLAN**

SCALE: 1/4" = 1'-0"

**FOOTING SCHEDULE**  
2000 PSF SOIL BEARING CAPACITY  
CONTACT ENGINEER IF OTHER SOIL CONDITIONS ARE ENCOUNTERED

WALL FOOTINGS	SQUARE FOOTINGS
W12E	TS16 8" x 16"
W12F	TS18 8" x 18"
W10E	TS20 8" x 20"
W10F	TS22 8" x 22"
W8E	TS24 8" x 24"
W8F	TS26 8" x 26"
W6E	TS28 8" x 28"
W6F	TS30 8" x 30"
W4E	TS32 8" x 32"
W4F	TS34 8" x 34"
W2E	TS36 8" x 36"
W2F	TS38 8" x 38"
W1E	TS40 8" x 40"
W1F	TS42 8" x 42"
W0E	TS44 8" x 44"
W0F	TS46 8" x 46"
W0E	TS48 8" x 48"
W0F	TS50 8" x 50"
W0E	TS52 8" x 52"
W0F	TS54 8" x 54"
W0E	TS56 8" x 56"
W0F	TS58 8" x 58"
W0E	TS60 8" x 60"
W0F	TS62 8" x 62"
W0E	TS64 8" x 64"
W0F	TS66 8" x 66"
W0E	TS68 8" x 68"
W0F	TS70 8" x 70"
W0E	TS72 8" x 72"
W0F	TS74 8" x 74"
W0E	TS76 8" x 76"
W0F	TS78 8" x 78"
W0E	TS80 8" x 80"
W0F	TS82 8" x 82"
W0E	TS84 8" x 84"
W0F	TS86 8" x 86"
W0E	TS88 8" x 88"
W0F	TS90 8" x 90"
W0E	TS92 8" x 92"
W0F	TS94 8" x 94"
W0E	TS96 8" x 96"
W0F	TS98 8" x 98"
W0E	TS100 8" x 100"

**PIER FOOTINGS**  
PIER = DIA. x T  
P12 = 12" x 12"  
P14 = 14" x 12"  
P18 = 18" x 12"  
P20 = 20" x 12"  
P22 = 22" x 12"  
P24 = 24" x 12"

**STEEL COLUMNS**  
P311 3" Ipa ADJ. STL. COL.  
P351 3 3/4" Ipa ADJ. STL. COL.  
P411 4" Ipa ADJ. STL. COL.  
P340 3" SCHED 40 PIPE COL.  
P360 3 1/2" SCHED 40 PIPE COL.  
P440 4" SCHED 40 PIPE COL.  
P540 5" SCHED 40 PIPE COL.  
P640 6" SCHED 40 PIPE COL.

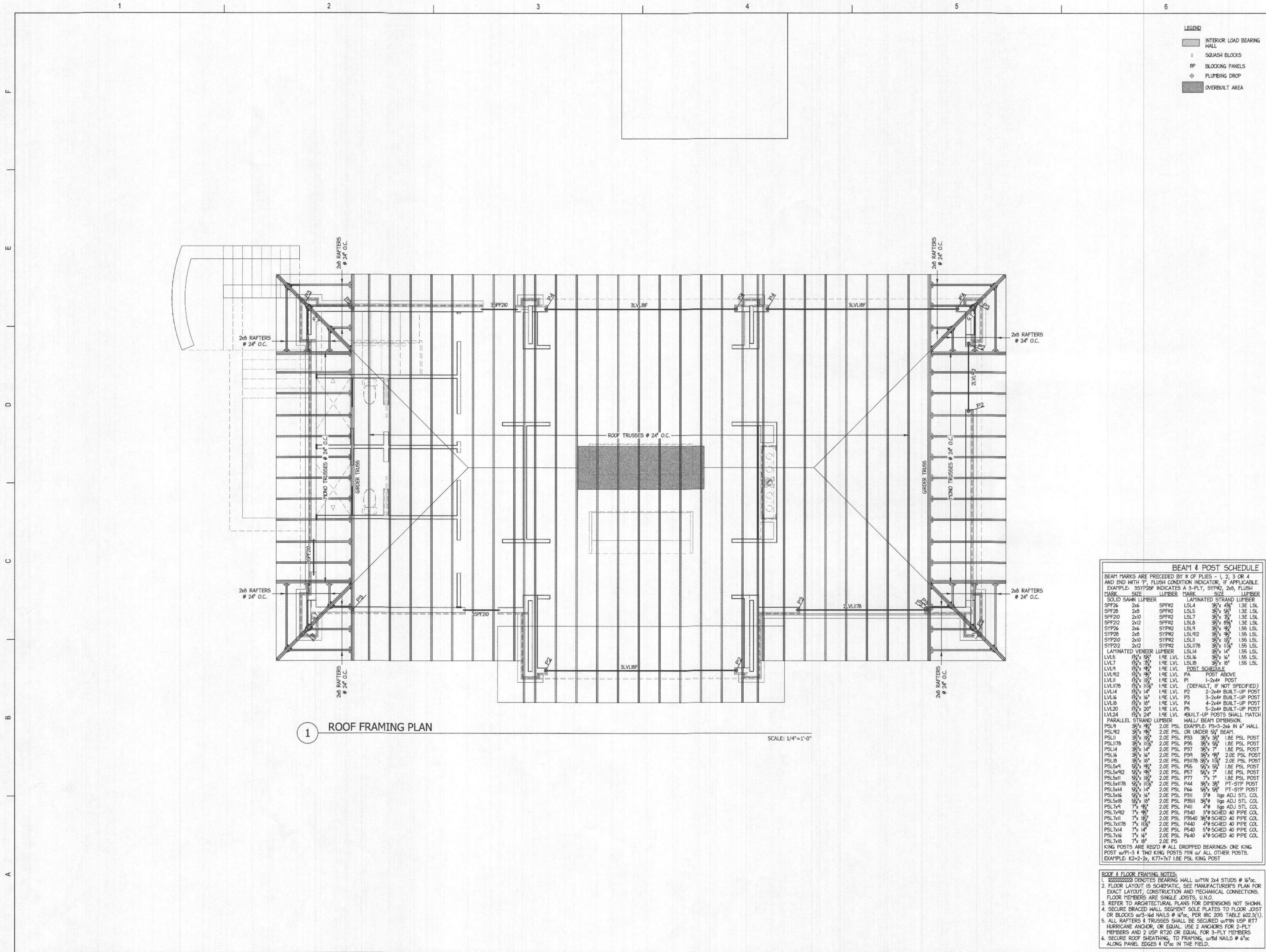
**PIER FOOTINGS**  
F12 = 12" x 12"  
F14 = 14" x 12"  
F18 = 18" x 12"  
F20 = 20" x 12"  
F22 = 22" x 12"  
F24 = 24" x 12"

**SQUARE FOOTINGS**  
F12 = 12" x 12"  
F14 = 14" x 12"  
F18 = 18" x 12"  
F20 = 20" x 12"  
F22 = 22" x 12"  
F24 = 24" x 12"  
F26 = 26" x 12"  
F28 = 28" x 12"  
F30 = 30" x 12"  
F32 = 32" x 12"  
F34 = 34" x 12"  
F36 = 36" x 12"  
F38 = 38" x 12"  
F40 = 40" x 12"  
F42 = 42" x 12"  
F44 = 44" x 12"  
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F72 = 72" x 12"  
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F78 = 78" x 12"  
F80 = 80" x 12"  
F82 = 82" x 12"  
F84 = 84" x 12"  
F86 = 86" x 12"  
F88 = 88" x 12"  
F90 = 90" x 12"  
F92 = 92" x 12"  
F94 = 94" x 12"  
F96 = 96" x 12"  
F98 = 98" x 12"  
F100 = 100" x 12"

N = NUMBER #4 BOTTOM BARS, EACH WAY

**FOUNDATION NOTES:**  
1. DENOTES BEARING WALL w/ MIN 2x4 STUDS @ 16"oc.  
2. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS NOT SHOWN.  
3. MIN. TREATED 2x6 SILL PLATE SHALL BE SECURED TO FOUNDN w/ MIN 1/2" x 4" ANCHOR BOLTS @ 6'-0"oc PER IRC 2015 R403.1.6.





**LEGEND**

- ▨ INTERIOR LOAD BEARING WALL
- SQUASH BLOCKS
- BP BLOCKING PANELS
- ⊕ PLUMBING DROP
- ▭ OVERBUILT AREA

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ARCHITECT: LEVINBROWN & ASSOCIATES, INC.  
 CONTRACTOR: J PAUL BUILDERS

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**BEAM & POST SCHEDULE**

BEAM MARKS ARE PRECEDED BY # OF FLIES - 1, 2, 3 OR 4 AND END WITH 'F'. FLUSH CONDITION INDICATOR, IF APPLICABLE. EXAMPLE: 3SYP20F INDICATES A 3-PLY, SYP#2, 2x6, FLUSH

MARK	SIZE	LUMBER	MARK	SIZE	LUMBER
<b>SOLID SAWN LUMBER</b>					
SPP26	2x6	SPP#2	L5L4	3x6	L3E L5L
SPP28	2x6	SPP#2	L5L5	3x6	L3E L5L
SPP20	2x10	SPP#2	L5L7	3x6	L3E L5L
SPP212	2x12	SPP#2	L5L8	3x6	L3E L5L
SYP26	2x6	SYP#2	L5L9	3x6	L5E L5L
SYP28	2x6	SYP#2	L5L12	3x6	L5E L5L
SYP210	2x10	SYP#2	L5L11	3x6	L5E L5L
SYP212	2x12	SYP#2	L5L178	3x6	L5E L5L
<b>LAMINATED VENEER LUMBER</b>					
LVL5	1 7/8" x 5 1/2"	1.9E LVL	L5L16	3x6	L5E L5L
LVL7	1 7/8" x 7"	1.9E LVL	L5L8	3x6	L5E L5L
LVL9	1 7/8" x 9"	1.9E LVL	<b>POST SCHEDULE</b>		
LVL#2	1 7/8" x 2"	1.9E LVL	PA	POST ABOVE	
LVL#1	1 7/8" x 1"	1.9E LVL	P1	1-2x4" POST	
LVL#178	1 7/8" x 178"	1.9E LVL	(DEFAULT, IF NOT SPECIFIED)		
LVL#4	1 7/8" x 4"	1.9E LVL	P2	2-2x4" BUILT-UP POST	
LVL#6	1 7/8" x 6"	1.9E LVL	P3	3-2x4" BUILT-UP POST	
LVL#8	1 7/8" x 8"	1.9E LVL	P4	4-2x4" BUILT-UP POST	
LVL#20	1 7/8" x 20"	1.9E LVL	P5	5-2x4" BUILT-UP POST	
LVL#24	1 7/8" x 24"	1.9E LVL	#BUILT-UP POSTS SHALL MATCH PARALLEL STRAND LUMBER HALL/ BEAM DIMENSION. EXAMPLE: P3-3-2x4 IN 6" WALL OR UNDER 5/2" BEAM.		
PSL#	3x6	2.0E PSL	P33	3x6	1.8E PSL POST
PSL#12	3x6	2.0E PSL	P35	3x6	1.8E PSL POST
PSL#178	3x6	2.0E PSL	P37	3x6	1.8E PSL POST
PSL#4	3x6	2.0E PSL	P39	3x6	2.0E PSL POST
PSL#6	3x6	2.0E PSL	P3178	3x6	118" 2.0E PSL POST
PSL#16	3x6	2.0E PSL	P55	3x6	1.8E PSL POST
PSL#42	3x6	2.0E PSL	P57	3x6	1.8E PSL POST
PSL#11	3x6	2.0E PSL	P77	7x7	1.8E PSL POST
PSL#178	3x6	2.0E PSL	P44	3x6	3x6 PT-SIP POST
PSL#44	3x6	2.0E PSL	P66	3x6	PT-SIP POST
PSL#16	3x6	2.0E PSL	P31	3" x 11ga	ADJ STL COL
PSL#18	3x6	2.0E PSL	P3511	3x6	11ga ADJ STL COL
PSL#4	7x14	2.0E PSL	P411	4" x 11ga	ADJ STL COL
PSL#42	7x14	2.0E PSL	P340	3" x 11ga	SCHED 40 PIPE COL
PSL#11	7x11	2.0E PSL	P3540	3x6	SCHED 40 PIPE COL
PSL#178	7x11	2.0E PSL	P440	4" x 11ga	SCHED 40 PIPE COL
PSL#44	7x14	2.0E PSL	P540	5" x 11ga	SCHED 40 PIPE COL
PSL#16	7x16	2.0E PSL	P640	6" x 11ga	SCHED 40 PIPE COL
PSL#18	7x18	2.0E PSL			

KING POSTS ARE REQ'D @ ALL DROPPED BEARINGS. ONE KING POST w/PH-3 & TWO KING POSTS MIN w/ ALL OTHER POSTS. EXAMPLE: K2-2-2x, K77-7x7 1.8E PSL KING POST

**1 ROOF FRAMING PLAN**

SCALE: 1/4"=1'-0"

SCALE: AS NOTED  
 DRAWN BY: SL CHECKED BY: JAU

ISSUE: DATE:  
 ISSUED FOR PERMITS 03-10-2021

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**ROOF FRAMING PLAN**

**S006**